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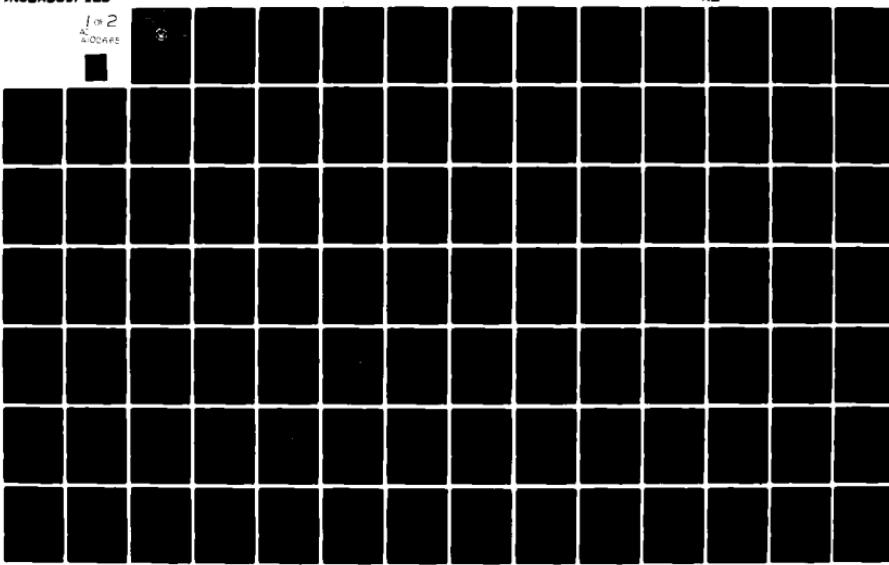
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NAVAL POSTGRADUATE SCHOOL

Monterey, California



THESIS

AN ELASTIC-PLASTIC FINITE ELEMENT ANALYSIS
OF NOTCHED ALUMINUM PANELS

by

Michael John Kaiser

March 1981

Thesis Advisor:

G. H. Lindsey

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Comparisons were made to the elastic, analytic series solution by Howland for a circular hole in a finite strip. The finite element results varied by less than one percent from Howland's solution. Handbook values for the elastic stress concentration factors of the geometries investigated differ from finite element results by less than one percent in all cases. The photoelastic works of Frocht were also compared where applicable. Stresses in the plastic range obtained from slip-line theory for a rigid-perfectly-plastic material show excellent correlation to a finite element analysis of such a material. Comparisons to elastic and plastic experimental data were made for the panels analyzed and show good correlation to finite element results.

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An Elastic-Plastic Finite Element Analysis
of Notched Aluminum Panels

by

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Lieutenant Commander, United States Navy
B.S., St. Cloud State University, 1969

Submitted in partial fulfillment of the
requirements for the degree of

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ABSTRACT

Finite element, elastic and plastic analyses of various aluminum panels, containing holes and notches, were conducted for comparison with photoelastic experimental results. A FORTRAN IV program, ADINA (Automatic Dynamic Incremental Nonlinear Analysis), was used for both linear and nonlinear analyses. Mesh refinements were used for each panel and the monotonically convergent results were extrapolated using Richardson's method. Stresses were locally smoothed from the Gauss integration points to the nodal points. Eight noded, isoparametric elements were used throughout. Modification to an ADINA preprocessor program, also coded in FORTRAN IV, was made for use with a VERSATEC plotter.

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SYMBOLS AND ABBREVIATIONS

ADINA	Automatic Dynamic Incremental Nonlinear Analysis
b	Half width of strip
CPU	Central processor unit
E	Young's Modulus of Elasticity
E_t	Strain hardening tangent modulus
FEA	Finite element analysis
JCL	Job control language
K_T	Stress concentration factor referenced to reduced cross-section σ/σ_n
MVS	Multiple virtual storage
n	Ramberg-Osgood exponent
$O(h^m)$	Order of the discretization error
r	Radius of hole or notch
VM	Virtual machine
β	Ramberg-Osgood coefficient
ϵ	General representation for strain
λ	Non-dimensional size parameter $\lambda=r/b$
ν	Poisson's Ratio of transverse strain
σ	General representation for stress
σ_θ	Principle stress in θ direction (hoop stress)
σ_r	Principle stress in radial direction
σ_n	Nominal stress in reduced cross-section
σ_∞	Far-field stress
σ_y	Yield stress by 0.2% offset method

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I. INTRODUCTION

Development of on-board fatigue monitoring systems for Naval aircraft have made it possible to record extensive structural loading data in flight. The strain gages used in such a system must be located away from stress concentration areas to prevent their fatigue; however, these areas are of the greatest interest in analyzing and predicting fatigue life of the structure. Understanding the relationship between nominal, far-field stresses and local stresses in critical areas thus becomes vitally important. Recent experimental investigations into the effect of uniform, far-field loads on stress concentration areas have been made at the Naval Postgraduate School (NPS) using photoelastic techniques [Refs. 1, 2 and 3]. These experiments involved loading 7075-T6 aluminum into both the elastic and plastic regions, as well as measurements of residual stresses resulting from plastic yielding.

Finite element analyses (FEA) of the aluminum panels used in the experiments of Stenstrom [Ref. 1] were conducted. The panels used in the experiments of Engle [Ref. 2] and Stuart [Ref. 3] have similar geometry. The finite element programs available at NPS were surveyed and ADINA [Ref. 4] was chosen for its proven ability to produce the nonlinear analyses required for plastic

yielding of aluminum. To provide increased accuracy, each panel was modeled using two meshes. The results obtained for the coarse and fine meshes were extrapolated to a final result using the Richardson extrapolation technique [Refs. 5 and 6].

Along with ADINA, a preprocessor program, PSAP1 [Ref. 7], was used to verify mesh connectivity prior to analysis by ADINA. PSAP1 provides a graphical output of the finite element mesh and was coded for the CALCOMP plotter installed at NPS prior to 1978. For this thesis PSAP1 was adapted for use with the VERSATEC plotter now installed at NPS.

The stress-strain material properties of the 7075-T6 aluminum actually used to make the panels had to be established to provide an accurate material model for use with ADINA. Material testing was conducted to establish the Young's Modulus (E), Poisson's Ratio (ν), yield stress (σ_y), strain hardening modulus (E_t) and the Ramberg-Osgood coefficients β and n .

Comparisons were made to other works, in addition to the experiments conducted at NPS. The initial analysis involved a comparison of FEA to the results of Howland [Ref. 8], for a circular hole in a finite strip, to validate the methods used. A comparison of FEA to plane stress, slip-line theory, for rigid-perfectly-plastic material was also included as a validation for the plastic analyses.

II. MATERIAL PROPERTY TESTING OF 7075-T6 ALUMINUM

The elastic and plastic material properties of the aluminum panels were established by tensile tests of uniaxial specimens made from the same mill run. The specimens were manufactured and tested according to current ASTM standards [Ref. 9]. MICRO-MEASUREMENTS, EA-13-125AD-120, precision strain gages with a temperature compensated bridge circuit were used on all specimens. Transverse gage sensitivity errors were corrected according to the manufacturer's recommendations [Ref. 10]. Critical cross-section measurements were made with a micrometer.

A. TESTS FOR AXIAL LOADING

Initial tests of the two-gaged specimen, Fig. 1, in the MTS testing machine indicated a significant bending moment was being produced by the 30,000 lb GRIFF grips. To investigate this problem further, tests were conducted on both the MTS and RIEHLE test machines with a five-gaged specimen shown in Fig. 2. The results of these axial loading tests, shown in Table I, verified that the GRIFF grips on the MTS test machine do not give axial loading. An inspection of the gripped region on the specimen indicated that the jaws of the grip were not applying a uniformly distributed force and thereby induced a bending

moment by off-axis loading as shown in Fig. 2. The grips on the RIEHLE test machine gripped evenly and a uniform strain distribution resulted as seen in Table I.

B. CHARACTERISTICS OF 7075-T6 ALUMINUM PANELS

The following characteristic properties were determined from the four specimens tested.

1. Young's Modulus (E)

Tests were conducted using the specimen shown in Fig. 3 on the MTS test machine with 10,000 lb INSTRON grips, which gripped the specimen evenly. The results of testing three specimens are shown in Tables II to IV. Linear regression in the elastic range of all the test results determined a Young's Modulus of 10.12×10^6 psi, with a correlation coefficient of 0.9996.

2. Poisson's Ratio (v)

Tests were conducted using the specimen shown in Fig. 1 on the RIEHLE test machine with 10,000 lb RIEHLE grips. The results are tabulated in Table V. Linear regression of these results in the elastic region determined Poisson's Ratio to be 0.3256 with a correlation coefficient of 0.99996.

3. Yield Stress and Strain Hardening Modulus

These values, required for ADINA's bi-linear material model, were determined graphically using the data from Tables III and IV. Plastic region data in

Table II is not reliable because of excessive creep encountered during that test.

0.2% offset yield stress, $\sigma_y = 76,000$ psi

strain hardening modulus, $E_t = 566,000$ psi

The graphical fit of these values to the test data can be seen in Fig. 4.

4. Ramberg-Osgood Coefficients

The Ramberg-Osgood equation for elastic-plastic stress-strain characterization is given by:

$$\epsilon = \frac{\sigma}{E} + \beta \left(\frac{\sigma}{E}\right)^n \quad (1)$$

where:

ϵ = strain

σ = stress

E = Young's modulus.

The β and n coefficients were determined graphically from the data of Table IV, by the method given by Rivello [Ref. 11]. The data in Table IV gave the following values which are the best fit to the combined test data

$$\beta = 1.479 \times 10^{43}$$

$$n = 21.58$$

The graphical fit of these values, in Eq. (1), with the test data is also shown in Fig. 4.

III. MODIFICATION TO GRAPHICAL PREPROCESSOR

The use of a graphical preprocessor program, such as PSAP1, is vital in detecting mesh errors that may otherwise go unnoticed. Establishing the proper node locations and element geometry prior to analysis for a complex code such as ADINA is of utmost importance.

A. PSAP1 MODIFICATIONS

The program PSAP1 was originally coded in FORTRAN IV for use on the NPS IBM 360/370 installation with the CALCOMP Model 765 drum plotter. The CALCOMP system originally installed at NPS used the +Y axis as the unlimited plotting axis, see Fig. 5. The entire plotting logic in PSAP1 uses this orientation of axes to allow multiple plots in a continuous strip. With the VERSATEC Model 8222A electrostatic plotter now installed at NPS, the +X axis becomes the unlimited plotting axis, shown in Fig. 5. To avoid an extensive recoding of PSAP1 for use with the VERSATEC plotter, a simple coordinate transformation of the plot was made in a limited number of short subroutines. First, all installation dependent plotting calls used in PSAP1 were identified. These involved seven plotter functions for which new subroutines were coded.

<u>Function</u>	<u>New Subroutine</u>
Initialize Plotter	CALCMP
Move Plotter Pen	CALPLT
Letter on Plot	NOTATE
Number on Plot	CALNUM
Determine Current Pen Location	CALWH
Draw a Line	CALINE
Stop Plotter	PSTOP

The subroutines listed above merely rotates the plot to coincide with the VERSATEC axis orientation and retain all features originally in PSAPI. Since all plotter hardware code is now isolated in these seven subroutines, future adaptations to other plotting systems is simplified. To provide documentation of this update to PSAPI, a complete listing of the new program is provided in Appendix D.

B. USE OF PSAPI

Previous use of PSAPI on the IBM 360 system necessitated use of a load module since PSAPI took over one minute of CPU time to compile. With the new IBM 3033 system compilation requires eight seconds; however, use of a load module or disk stored source code is still recommended since PSAPI contains roughly 2,500 lines of code. Appendix A contains sample JCL to use PSAPI on the IBM 3033 MVS system. With minimal effort PSAPI could also be set up

for use on the IBM VM/370 system. The user's manual for PSAP1 is in Ref. 7. In addition to a mesh plot, PSAP1 provides a listing of the node coordinates, element connectivity and several key input values used in execution of ADINA. This information provides a useful check of the input data.

IV. FINITE ELEMENT ANALYSIS (FEA)

A. DESCRIPTION OF MESHES USED

1. Length to Width Ratios and Boundary Conditions

Initial finite element models of specimens had length to width ratios near one, as used by Garske [Ref. 12] for his FEA, but they did not provide the desired uniform distribution at the loading boundary. Specimen length to width ratios of 3-5 were used by Armen, Pifko and Levine [Ref. 13] in their FEA and by Stenstrom [Ref. 1] in his photoelastic experiments. The criteria established to determine uniform boundary stress distribution was uniformity in nodal displacements along the loaded edge as discussed by Segerlind [Ref. 14]. In the models used for FEA in this thesis, nodal displacements were uniform to within 0.1%, and the resulting stress distribution was uniform axially to within 0.1% at the panel ends. In all cases two-dimensional, eight noded, isoparametric elements were used. These higher order elements cannot be loaded in an "intuitive" manner as discussed by Zienkiewicz [Ref. 15, p. 223]. Figure 6 shows the nodal loading required to obtain a uniform surface load.

2. Element Meshes

Two meshes were developed for each panel analyzed. A reasonable effort was made to keep element corner angles as close to 90° as possible to reduce the effect of element distortion discussed by Hopkins and Gifford [Ref. 16]. All meshes modeled a quarter of the actual panel by using the two axes of symmetry as is common practice in FEA. The step from coarse to fine element meshes was made so that each element in the coarse mesh was subdivided into four smaller elements of the same type. Such a mesh subdivision can be expected to give monotonic convergence of results, Cook [Ref. 17], and allow extrapolation to results of an infinitely fine mesh. Figures 7 through 13 illustrate the element meshes used in this analysis as plotted by PSAP1.

B COMPUTATIONAL PROCEDURES

1. Using ADINA

Once the mesh has been developed, input data is prepared in accordance with the ADINA user's manual [Ref. 4]. This same set of data is then used as input for PSAP1 to check for errors and provide a graphical display of the element mesh. After preprocessing by PSAP1, the data is entered into ADINA for analysis. In the case of linear analysis, two types of stress output may be specified, nodal point or Gauss integration point. Nodal point output

can be computed for up to eight node point stresses for each element. Since 2x2 Gauss integration was used, four Gauss point stresses were computed for each element. The 2x2 Gauss integration is recognized as the most efficient integration order for this type of analysis [Ref. 15, p. 284]. The linear analysis used an isotropic linear elastic material model (MODEL "1" in Ref. 4) which required input of E and ν material properties. The nonlinear analysis allows only Gauss point stress outputs and uses a bilinear elastic-plastic material model, with von Mises yield condition and isotropic strain hardening (MODEL "8" in Ref. 4).

For static analyses ADINA uses a time function method to apply loads in steps. Linear analysis loading was accomplished in a single step to a nominal value of 3,000 lbs load. Nonlinear analysis loads were applied in ten steps to a maximum value, matching the experimental loads, and then unloaded to zero in ten steps to obtain residual stresses. The stress output from ADINA is a listing of nodal or Gauss point stresses for each element. Since the only area of interest in this analysis was the distribution of stresses along the reduced cross-section, no large post-processing program was developed or used. All final computations using ADINA output data were accomplished on a HEWLETT-PACKARD 9830A calculator, using short programs coded in BASIC. If more extensive stress

distribution information were desired, some form of automated post-processing would be necessary to reduce the computational workload. At a minimum, nodal stress outputs by ADINA must be averaged to obtain unique values of stress at nodes shared by more than one element.

2. Richardson Extrapolation

The use of course and fine meshes allows extrapolation to an infinitely fine mesh as discussed earlier.

Richardson extrapolation [Ref. 5] was used in this analysis where:

$$\sigma_{\text{extrap}} = \frac{\sigma_C(h_F)^m - \sigma_F(h_C)^m}{h_F^m - h_C^m} \quad (2)$$

where

σ_{extrap} = extrapolated solution

σ_C = solution obtained with $h=h_C$

σ_F = solution obtained with $h=h_F$

h_C = linear dimension of course element

h_F = linear dimension of fine element

m = 2 (for this analysis)

The exponent m is determined by the order of the discretization error $O(h^m)$. Since h represents the length of an element the element area is represented h^2 . In a two dimensional problem such as this $O(h^m)$ is of the order of h^2 , the area of an element. In the mesh refinement scheme

used $h_F = \frac{1}{2} h_C$ or $\frac{h_F}{h_C} = \frac{1}{2}$. Equation (2) can be rewritten

$$\sigma_{\text{extrap}} = \frac{\sigma_C \left(\frac{h_F}{h_C}\right)^2 - \sigma_F \left(\frac{h_C}{h_F}\right)^2}{\left(\frac{h_F}{h_C}\right)^2 - \left(\frac{h_C}{h_F}\right)^2} \quad (3)$$

thus

$$\sigma_{\text{extrap}} = \frac{\sigma_F - \frac{1}{4} \sigma_C}{\frac{3}{4}} \quad (4)$$

Equation (4) then becomes the relation to obtain extrapolated stresses from coarse and fine mesh results in a two dimensional analysis. Better extrapolations can be obtained by using three or more refined meshes, but, the computational effort increases significantly.

3. Optimal Stress Locations and Local Smoothing

It is generally accepted that the most accurate sampling points for stresses are the Gauss integration points within the element [Ref. 15, p. 281, and Ref 18]. In this analysis, the nodal points are of the greatest interest; thus a technique of local smoothing must be applied to the integration point stresses to obtain nodal stresses as reported by Hinton and Campbell [Ref. 19]. The formulation of this local smoothing technique for ADINA elements is developed in Appendix B. The nodal values obtained must then be averaged if shared by two or more elements.

4. Computational Times

Because of the extremely large size of ADINA (about 17,000 lines of code in the NPS version) and the out of core solver, it does not adapt well to time sharing systems. Using the IBM 360 system at NPS, ADINA required 31 user defined overlays to create a manageable load module in about 30 minutes of CPU time. With the new IBM 3033 MVS system at NPS, ADINA is compiled without overlays in about one minute. When using a load module, the program execution took less than 2 minutes CPU time on the IBM 3033.

In addition to ADINA, the preprocessor (PSAP1) and post-processing techniques involve considerable time and effort. Figure 14 is a flow chart of the computational procedure used in this analysis. An example of the JCL to use ADINA in load module form on the IBM 3033 MVS system with use of the mass storage facilities is shown in Appendix C.

V. RESULTS OF ANALYSIS

A. CIRCULAR HOLES IN LINEAR MATERIAL

The FEA results for a circular hole in a finite width strip were used to validate the elastic computational procedure discussed earlier. The results of Howland [Ref. 8] were compared to both the Gauss point smoothed results and the nodal output results in Fig. 15. The stress concentration factor σ/σ_∞ is referenced to the far-field stress.

The smoothed results give the best match to the results of Howland at the edge of the hole, and the only significant variation between the two FEA methods occurs within the first 0.25 inches from the edge. In order to obtain the 0.25 inch stress value for the coarse mesh, in the Gauss point smoothed result, a midside node value had to be obtained by the averaging method discussed in Appendix B. The linear distribution of smoothed stresses along the sides of the element, [Ref. 19], appears to produce a less accurate result in this area of extreme stress gradient, when compared to ADINA's nodal output result. This tendency was noted in all cases; however, the peak stress values from smoothed results consistently gave better correlation with other investigators [Ref. 20].

A circular hole with $\lambda=0.25$ was also analyzed and compared to the experimental results of Stenstrom [Ref. 1]

along with an interpolation of Howland's results. The σ_θ experimental data correlates well with the FEA results; however, the σ_r experimental data shows significant variation between 0.125 and 0.375 inches from the edge of the hole, as seen in Fig. 16.

B. OPPOSITE U NOTCHES IN LINEAR MATERIAL

The results for linear analysis are presented in non-dimensional stress concentration form; however, the normalizing stress changes. For U notches K_T is the stress concentration factor referenced to the theoretical, nominal stress (σ_n) in the reduced cross-section where $\sigma_n = \text{Load}/\text{Area of Reduced Cross-Section}$.

1. Shallow Notch Panel

The FEA results plotted with the experimental data of Stenstrom are shown in Fig. 17. Once again both FEA results are shown and the variation for the two methods occur within 0.25 inches from the notch edge; however, there was less variation than was seen in the circular hole analyses.

For this panel the experimental data appear to be uniformly below the FEA results for σ_θ . The σ_r data shows significant variation at the 0.625 inch point but follows the proper trend within 0.5 inches from the notch edge. According to data collected by Peterson [Ref. 20], this notch geometry should yield a maximum $K_T = 2.74$. The

Gauss point smoothed results matched this value exactly. Stuart [Ref. 3] reported a $K_T = 2.69$ for this same notch geometry, with a standard deviation of 0.187 in the 14 samples he measured by photoelastic methods. Early photoelastic work by Frocht [Ref. 21] determined $K_T = 2.7$ for this notch geometry. The FEA results appear to be in good agreement with other investigators, for this notch geometry.

2. Deep Notch Panel

Results of the FEA for a deep notch were plotted with Stenstrom's experimental data in Fig. 18. The results of the two FEA methods again diverge within 0.5 inches from the notch edge. FEA stress values at the 0.25 inch point have spread farther apart in this case since the stress gradient is very severe at that point. The experimental data correlates well for both σ_θ and σ_r ; however, the maximum experimental σ_θ is considerably lower with a $K_T = 3.83$. Results reported by Stuart for this notch geometry was a $K_T = 4.05$ with a standard deviation of 0.219 for 14 specimens measured by photoelastic methods. Frocht [Ref. 22] reported a photoelastic $K_T = 3.9$ for this notch geometry, but concluded that the result was 5-10% low, giving a corrected range of K_T from 4.1 to 4.3. The Gauss smoothed FEA result gave a $K_T = 4.24$ which compares well to an empirical relation given by Peterson [Ref. 20] for $r/d < 0.25$.

$$K_T = \left(1 - \frac{2t}{D}\right) \left(0.78 + 2.243\sqrt{t/r}\right) \left[0.993 + 0.18 \frac{2t}{D} - 1.06 \left(\frac{2t}{D}\right)^2 + 1.71 \left(\frac{2t}{D}\right)^3 \right] \quad (5)$$

where

t = notch depth (3.9375)

r = notch radius (0.625)

d = minimum width (15.625)

D = maximum width (23.5)

Inserting the values above into Eq. (5) gives a $K_T = 4.26$, which is 1/2% above the FEA result. Other FEA results reported by Armen, Pifko and Levin [Ref. 13] and Griffis [Ref. 23], using linear strain triangle (LST) elements, produced K_T values within 5% of those produced by use of Eq. (5). The rectangular elements used in this analysis are known to give better results than LST elements as noted by Clough [Ref. 24]. It is clear that the FEA results obtained for this notch are in good agreement with other works.

C. OPPOSITE U NOTCHES IN NONLINEAR MATERIAL

The analysis for loading into the plastic region of the 7075-T6 aluminum was made using the bilinear material model discussed earlier. The loads used were selected to match those used in the experiments of Stenstrom; thus allowing direct comparison. The strains obtained in those experiments were used to solve for stresses by use of the Prandtl-Reuss plastic flow equations.

1. Shallow Notch Panel

The results of FEA for the three load cases, 60,000, 65,000 and 70,000 lbs are presented along with the experimental results in Figs. 19 through 21. The σ_θ results compare well although no trend for peak σ_θ stress away from the notch edge is shown in the experimental data. In all cases the FEA determined the peak σ_θ stress to occur near the yield boundary, and the gradient of the σ_θ stress to fall off dramatically in the plastic zone. This characteristic behavior of the σ_θ stress was reported by other investigators [Refs. 13 and 23] using FEA on 2024-T3 aluminum. Plane elastic-plastic stress distributions reported by Frocht [Ref. 25] show similar trends. The experimental data also shows a marked change in the gradient of σ_θ stress within the plastic region. The growth of this plastic region is approximated using the FEA results for this notch in Figs. 22 through 24.

Experimental data for the σ_r stress distribution matches the FEA results closely except at the notch edge where the measured σ_r does not go to zero as it should. The characteristic peak value of σ_r near the plastic boundary as seen in the FEA results is also shown by the test data.

The FEA residual stress computed upon unloading from the three load cases are shown in Figs. 25 through 30. The characteristic distributions of the σ_θ residual stress

agrees with those reported by others [Refs. 25, 26 and 27]. The experimental residual stress distributions reported by Stenstrom show similar trends but significant variations when compared to the FEA results.

2. Deep Notch Panel

Three load cases were computed to plastic loading levels; however, only limited experimental results are available for this notch as seen in Fig. 31. The 30,000 lb load is just at the onset of yield in the notch root area. Limited residual experimental data [Ref. 3] was available for comparison in Figs. 32 and 33 which are plots of the residual σ_θ and σ_r stress distributions as a result of the three loading cases. The 30,000 lb load has caused yielding in a small region at the root of the notch as seen in Fig. 34. Figures 35 and 36 illustrate that the plastic zone does not grow to the extent it did in the shallow notch. The stress gradients are very severe in the deep notch and as a result the 0.25 inch sampling points used in the FEA may be useful in only showing gross trends close to the notch edge. The trends appear to be much the same as in the shallow notch, with the peak σ_θ and σ_r stresses occurring near the yield boundary. The yield boundaries shown in Fig. 31 are approximations based on qualitative analysis of the finite element results. The σ_θ experimental data for the 30,000 lb load case correlates well with the peak stress, again

appearing low as it did in the linear analysis. The residual σ_θ stress distribution in Fig. 32 follow much the same trends as seen in the shallow notch but with much higher gradients within the first 0.5 inches from the notch. The FEA results for the residual σ_r stresses shown in Fig. 33 indicate a limitation in the element size used since the σ_r value of the notch edge does not return to zero as it should. Because of this problem the data may be questionable for showing proper trends in the first 0.5 inch from the notch edge.

3. Rigid-Perfectly Plastic Panel

A stress distribution for the theoretical material used in slip-line theory was desired. By using ADINA's bilinear material model such a material could be approximated reasonably well. For this analysis a Young's Modulus (E) of 10^{26} psi was used to model perfect rigidity. The strain hardening modulus (E_t) was set to zero to model perfect plasticity. Poisson's ratio (ν) was 0.4999999, as close to 0.5 as the computer would allow. The $E = 10^{26}$ also represents a computer limit in approaching an infinitely large E . Figure 37 illustrates the results obtained and compares them to a slip-line solution. The results are normalized to the arbitrary 73,000 psi yield stress used in the analysis. The σ_θ values obtained agree exactly with slip-line theory. Conversely the σ_r results do not reflect the same values as slip-line theory, but do show

a similar trend. The growth of the plastic zone obtained is shown in Figs. 38 through 40.

VI. CONCLUSIONS AND RECOMMENDATIONS

The results obtained from the FEA have proven useful in determining the validity of experimental data gathered by photoelastic techniques. The FEA results varied by less than 1% when compared to published analytical results and handbook values. Experimental data from Stenstrom's photoelastic work correlated well with the FEA results. The primary exception was the residual stress experimental values, which varied significantly from the FEA results. The variation may be in transforming the residual strains measured photoelastically into stresses for comparison with the FEA results, since ADINA only provides a stress output. Limitations of ADINA's bilinear material model, initially considered severe, do not appear to have hampered this investigation. A possible exception is the residual analyses where the transition region from elastic to plastic strains becomes especially important. The Gauss point smoothed results gave the best correlations at the edge singularities in all cases; however, due to the limitations noted at 0.25 inches from the edge, the results at that specific point may not be as accurate for this method. The nodal output results gave consistently higher stress values at the edge singularities. Because

of the severe stress gradients near the deep notch analyzed the use of a finer element mesh near the notch would probably produce better results.

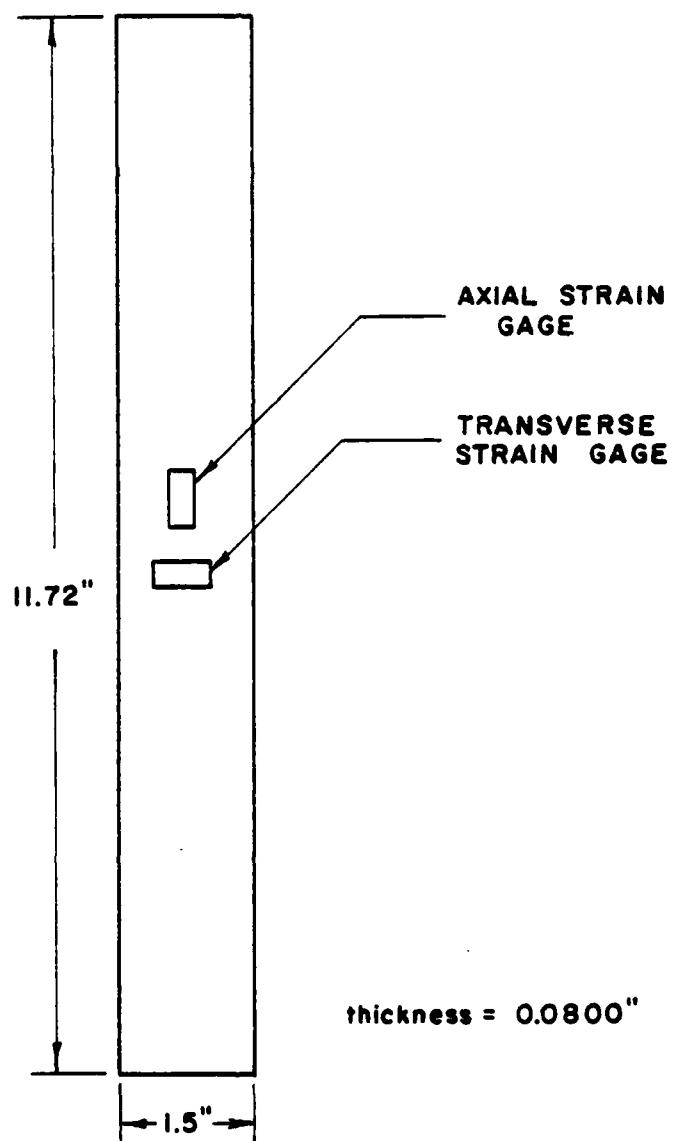
The effort involved in developing two meshes such as those used in this thesis is considerable. An automatic mesh generation capability would reduce the workload and allow experimentation with several types of element meshes.

ADINA proved to be a useful and powerful program, as expected, but something simpler and less awkward to use may be all that is required for two dimensional analysis. Such a system is already in use at NPS but does not offer non-linear capabilities. If use of ADINA is to be continued in this type of investigation, a post-processing program should be adapted. There are programs available to post-process ADINA data at NPS [Refs. 28 and 29] but they would require modifications to work with two dimensional analyses and the VERSATEC plotter.

Standardized material property testing would ease the inevitable task of obtaining basic material properties for use in analysis or experiments. Some form of automatic data collection with use of the MTS testing machine would allow testing of a larger sample population and provide statistically more accurate information.

FIGURE 1

2 GAGE SPECIMEN



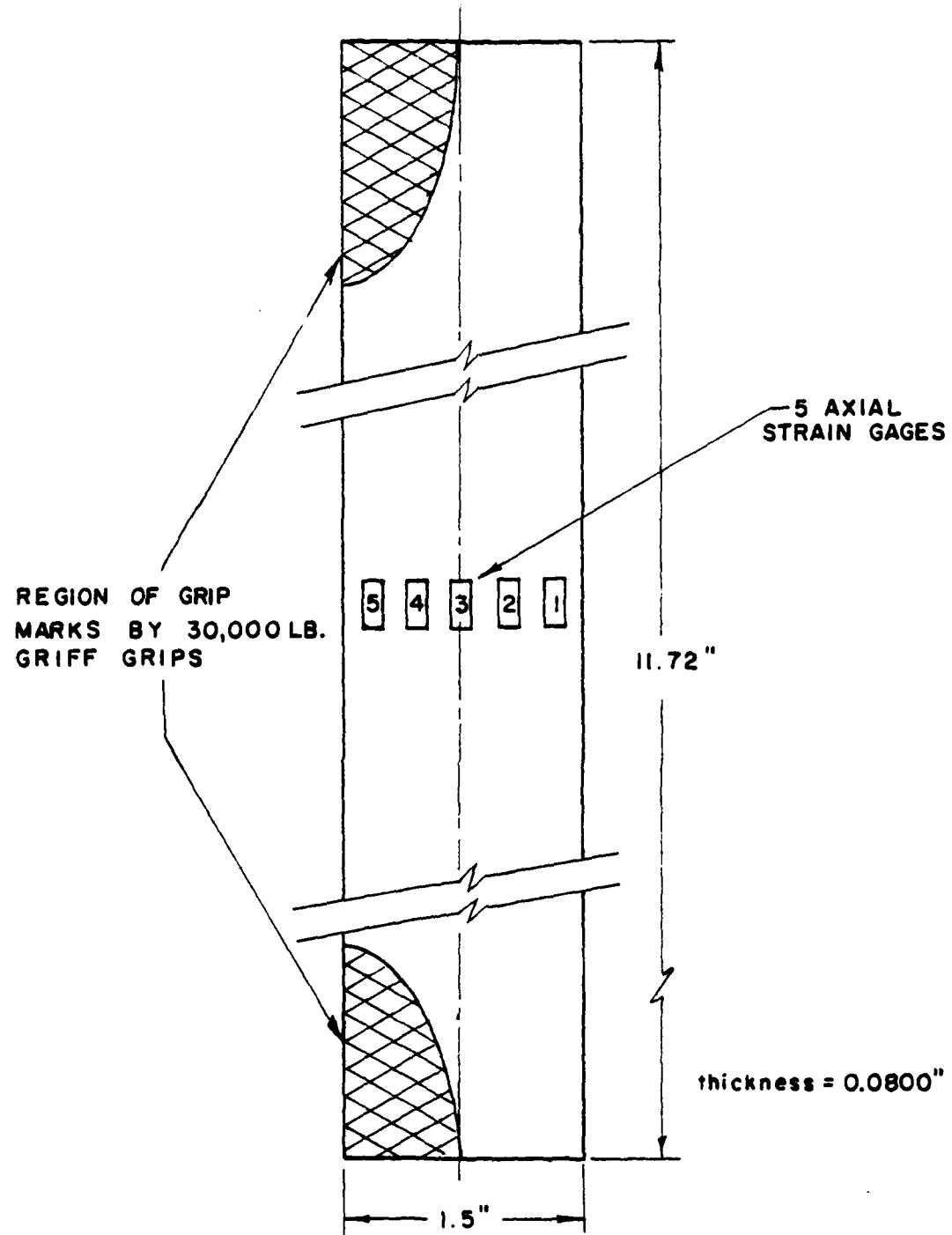


FIGURE 2

5 GAGE SPECIMEN

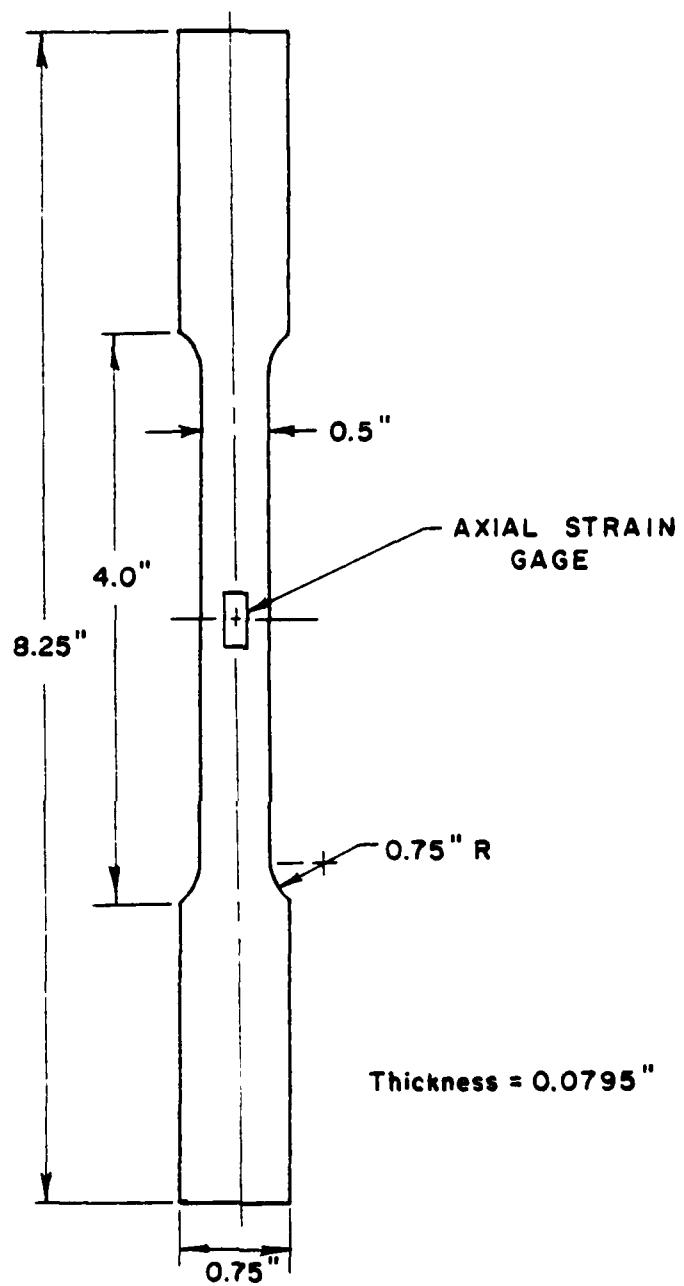
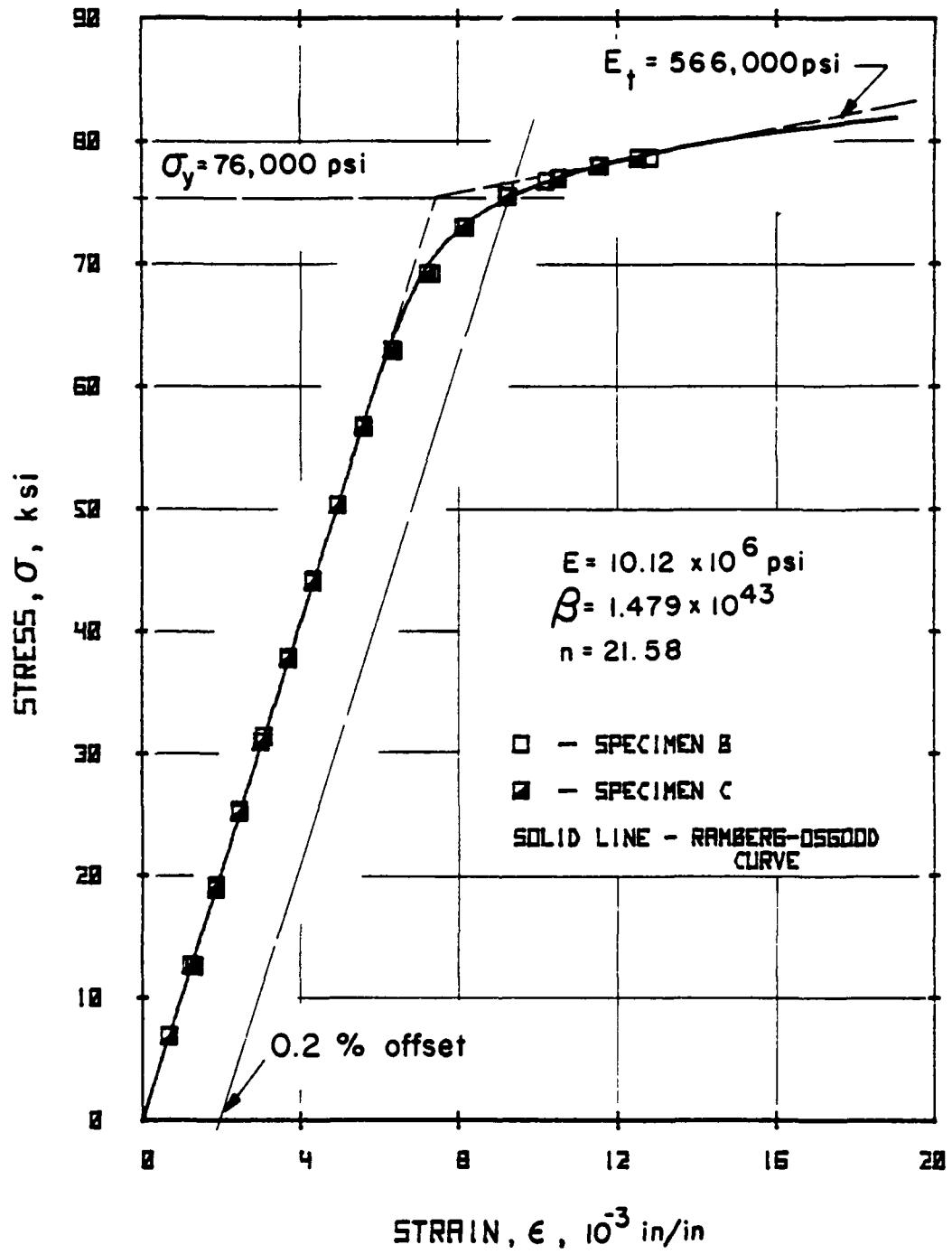


FIGURE 3

1 GAGE SPECIMEN

FIGURE 4
7075-T6 ALUMINUM STRESS-STRAIN CURVE



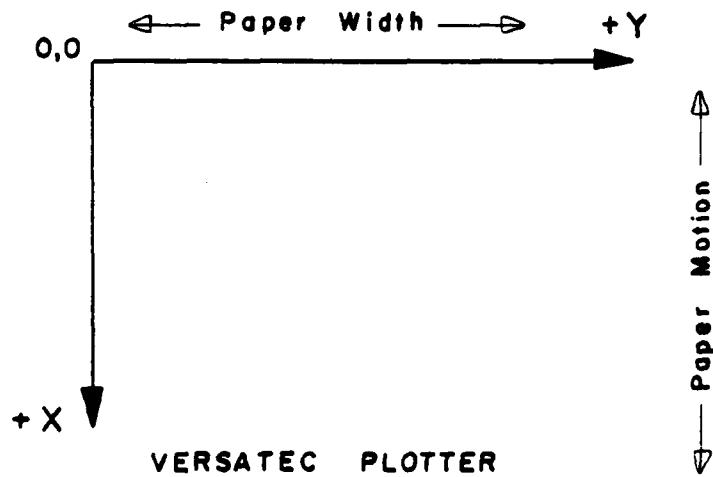
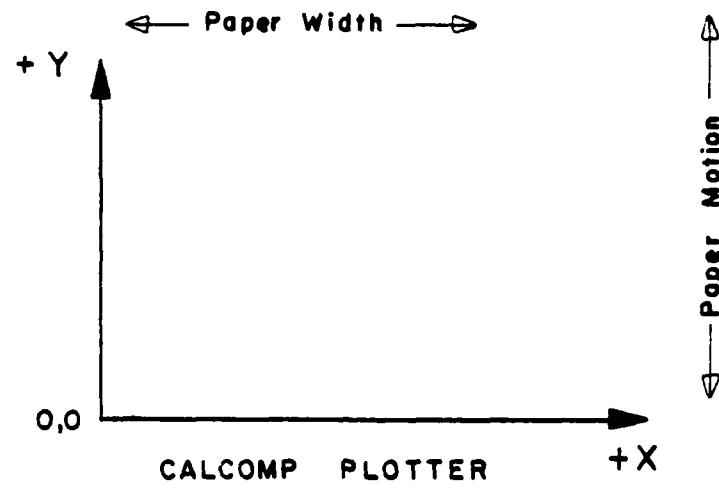


FIGURE 5

CALCOMP AND VERSATEC PLOTTER AXES

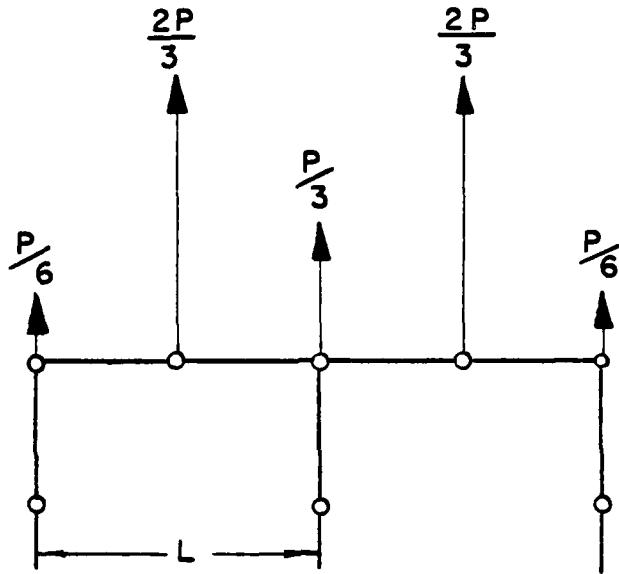
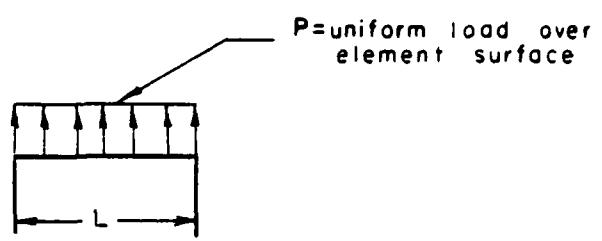


FIGURE 6

NODAL LOADING DIAGRAM

28 ELEMENTS (ISOPARAMETRIC)

111 NODES

192 DEGREES OF FREEDOM

DIMENSIONS

λ	W	L	RADIUS
0.2	5 "	25 "	1 "
0.25	4.0625 "	20 "	1 "

$$\lambda = \frac{\text{RADIUS}}{W}$$

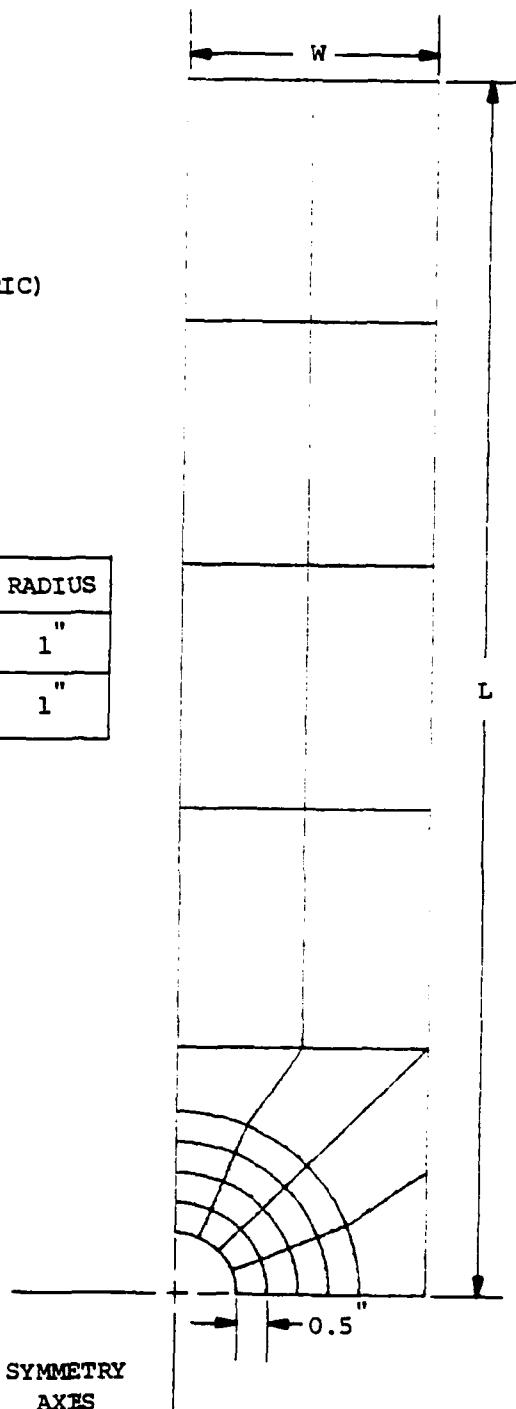


FIGURE 7

COURSE MESH FOR CIRCULAR HOLES

A SUBDIVISION OF COURSE MESH

112 ELEMENTS (ISOPARAMETRIC)

389 NODES

720 DEGREES OF FREEDOM

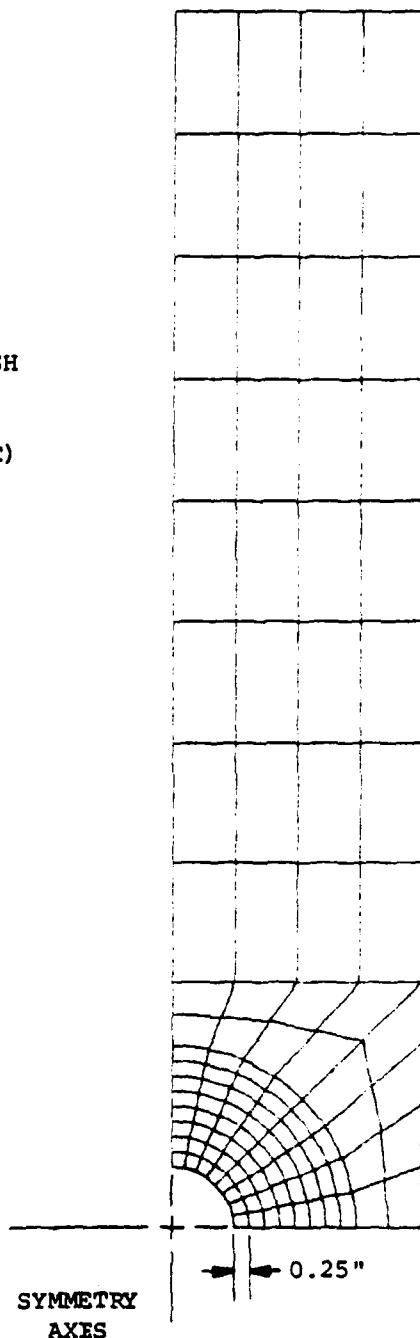


FIGURE 8

FINE MESH FOR CIRCULAR HOLES

FIGURE 9 COURSE MESH FOR SHALLOW NOTCH

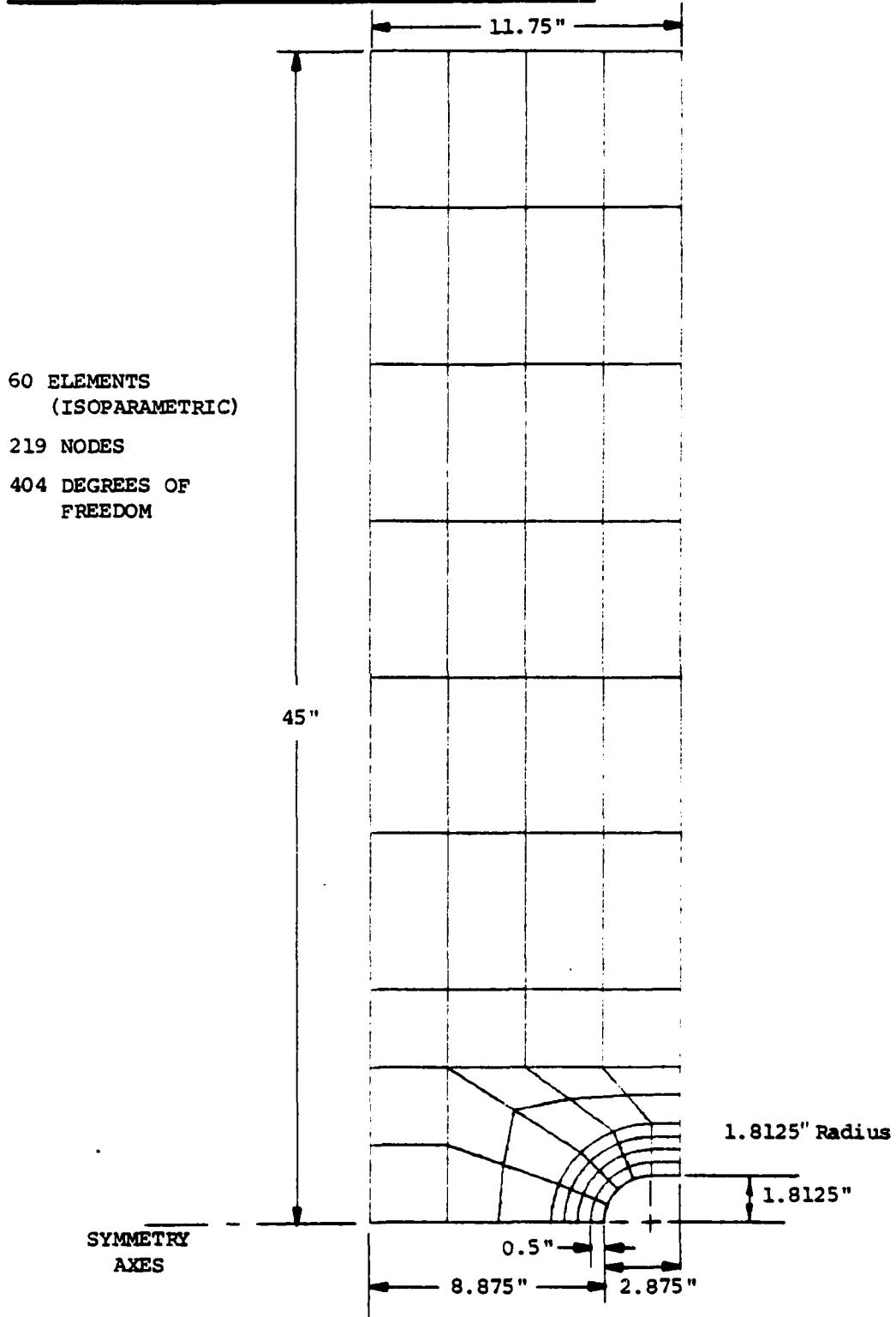


FIGURE 10 FINE MESH FOR SHALLOW NOTCH

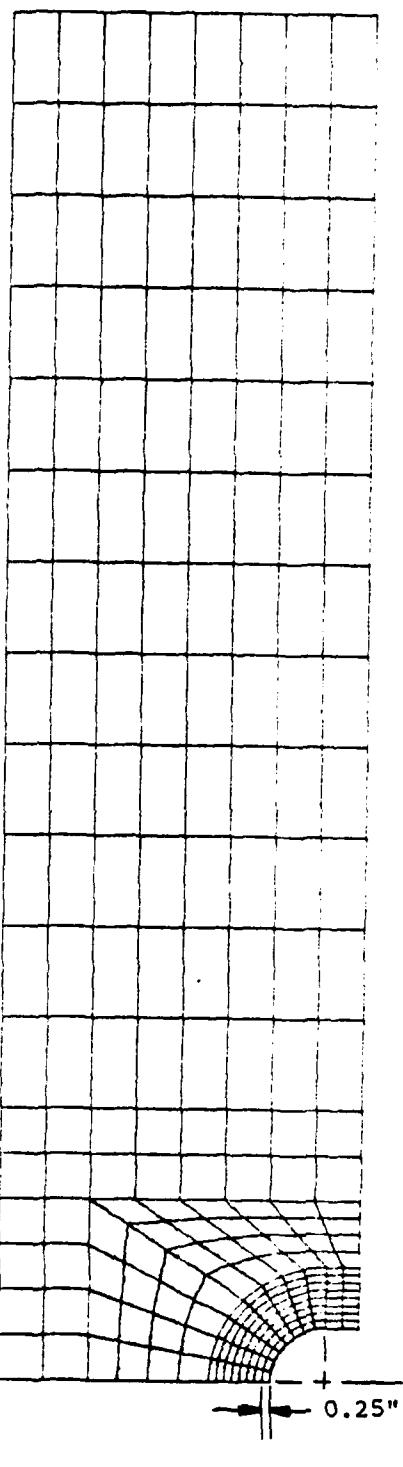
SUBDIVISION OF
COURSE MESH

240 ELEMENTS
(ISOPARAMETRIC)

797 NODES

1528 DEGREES OF
FREEDOM

SYMMETRY
AXES



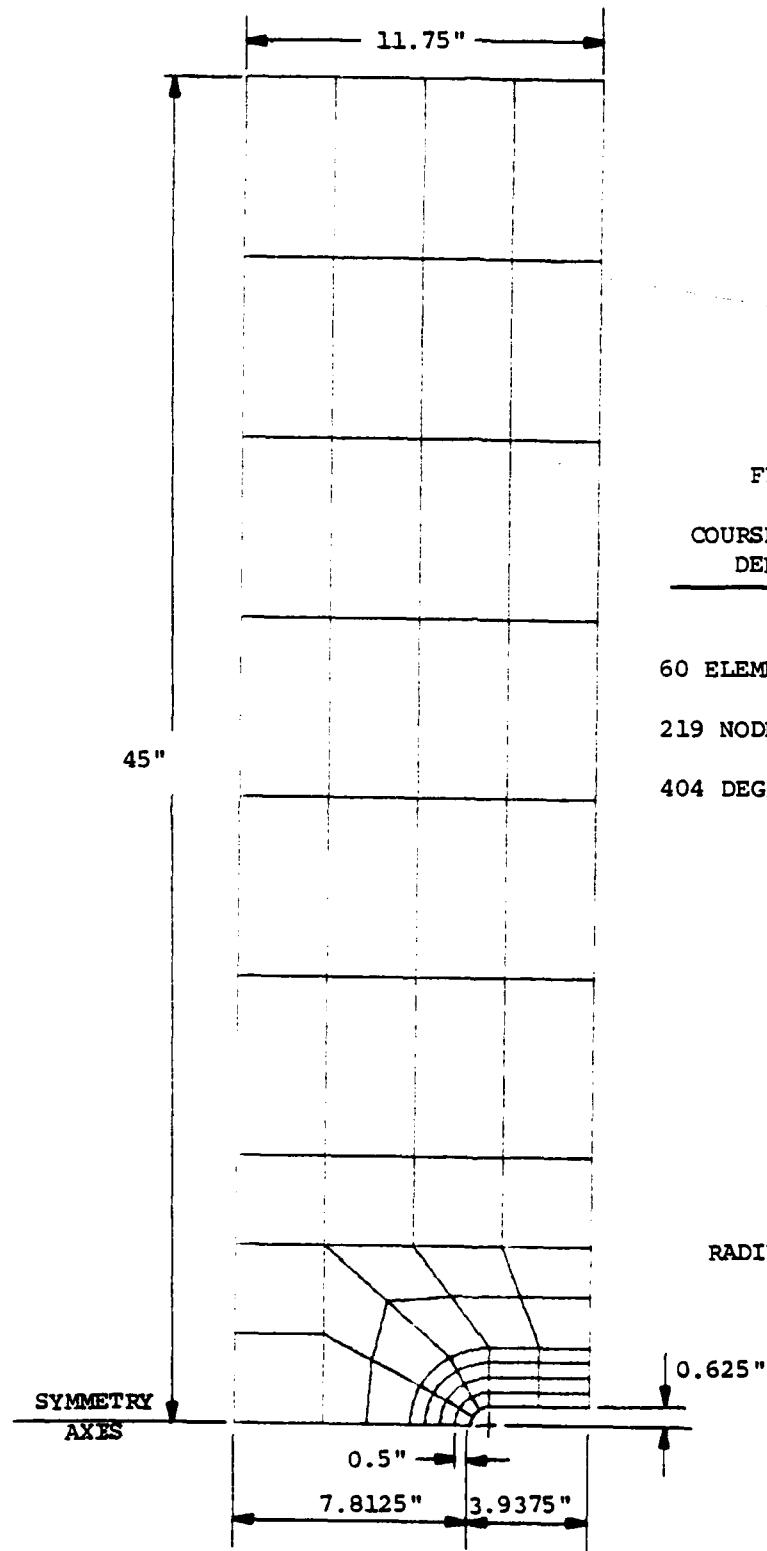


FIGURE 11

**COURSE MESH FOR
DEEP NOTCH**

60 ELEMENTS (ISOPARAMETRIC)

219 NODES

404 DEGREES OF FREEDOM

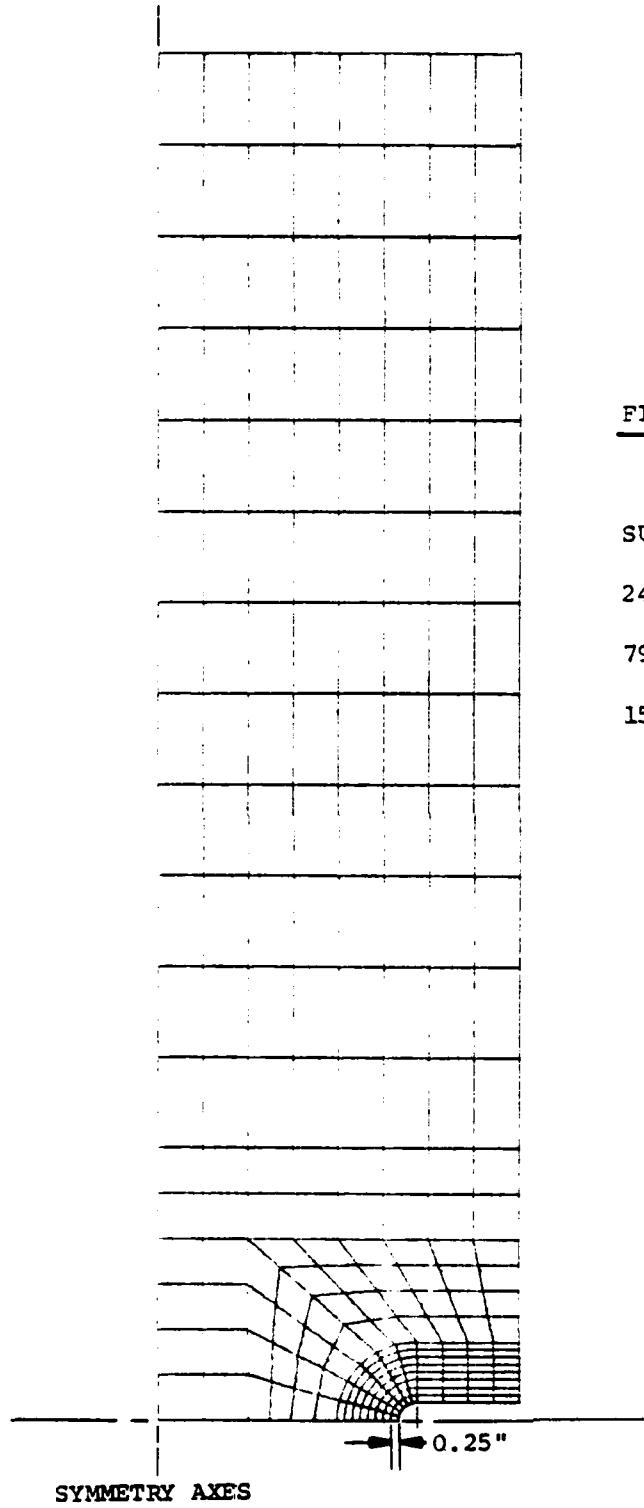


FIGURE 12
FINE MESH FOR DEEP NOTCH

SUBDIVISION OF COURSE MESH
240 ELEMENTS (ISOPARAMETRIC)
797 NODES
1528 DEGREES OF FREEDOM

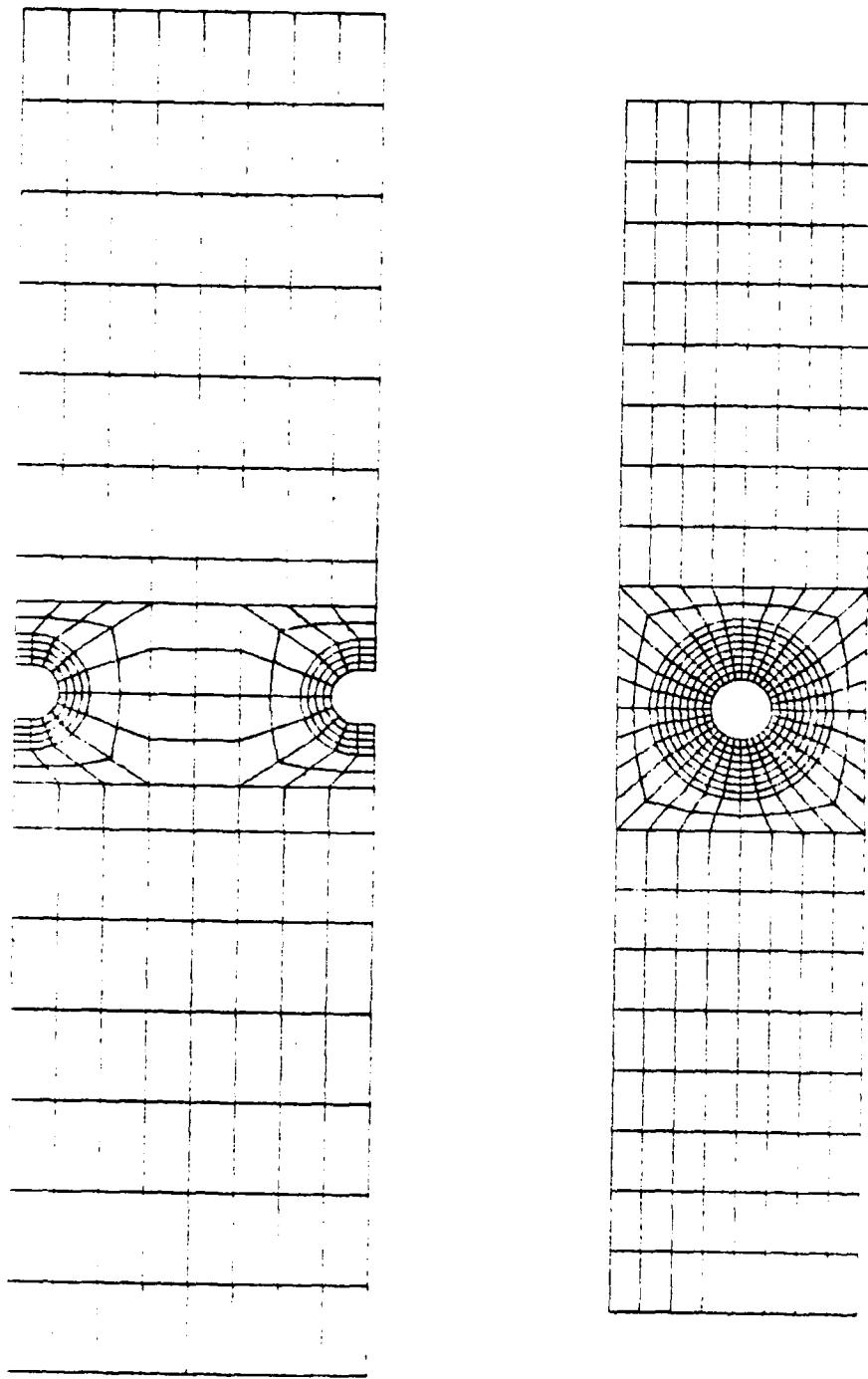


FIGURE 13

EXAMPLE OF COMPLETE PANEL MESHES

FIGURE 14

COMPUTATIONAL FLOW CHART

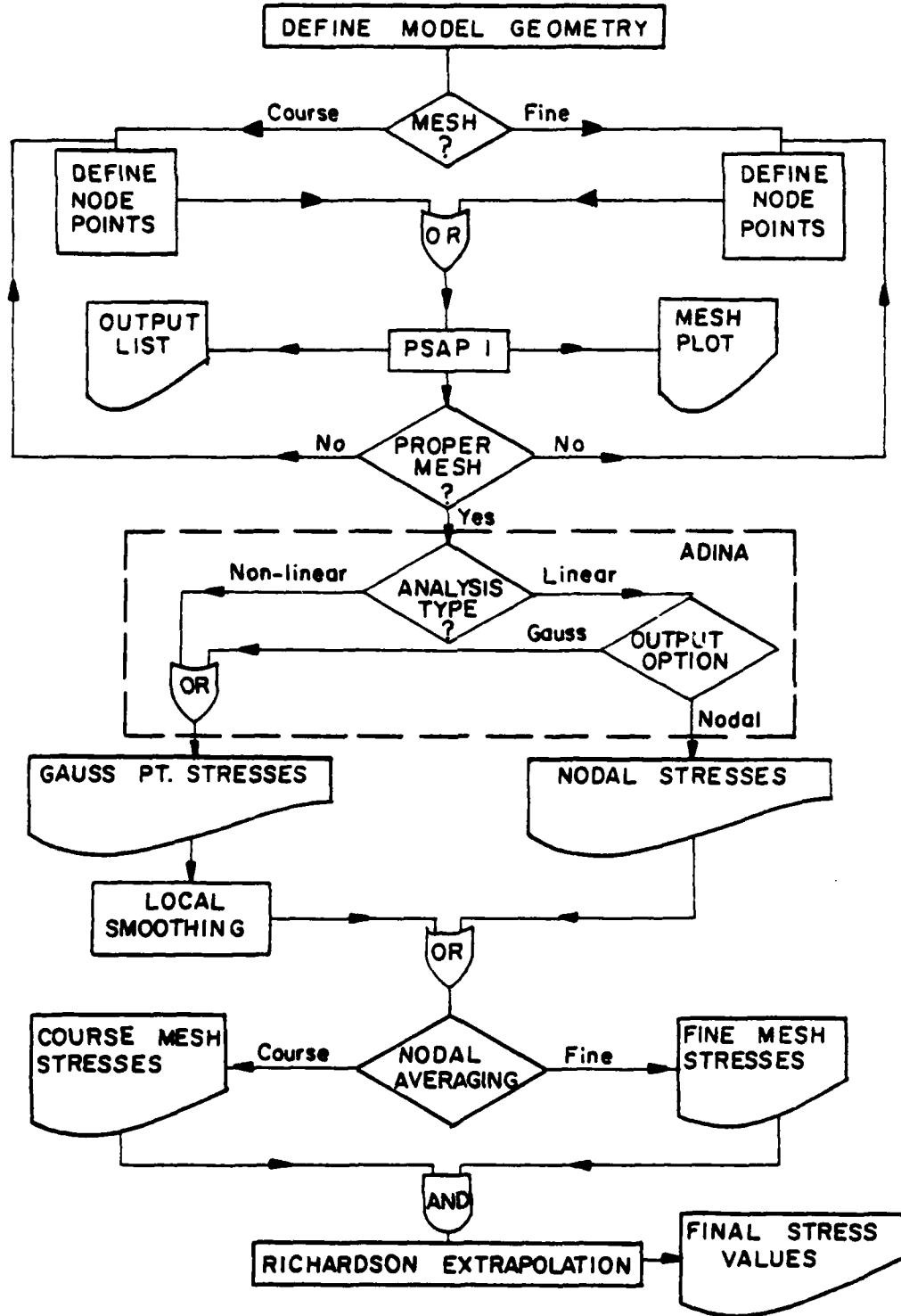


FIGURE 15

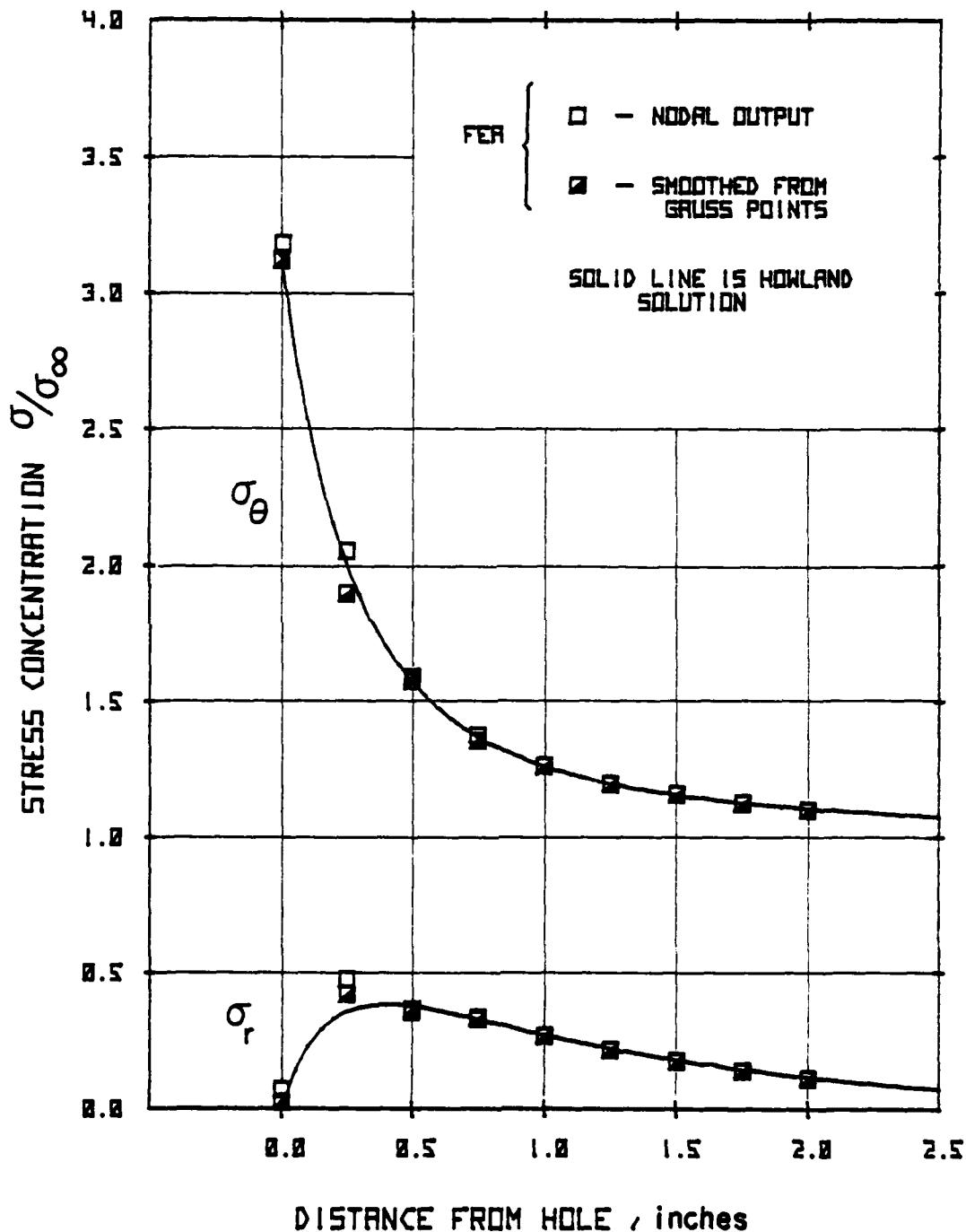
CIRCULAR HOLE $\lambda=0.2$ LINEAR RESULTS

FIGURE 16
CIRCULAR HOLE $\lambda=0.25$ LINEAR RESULTS

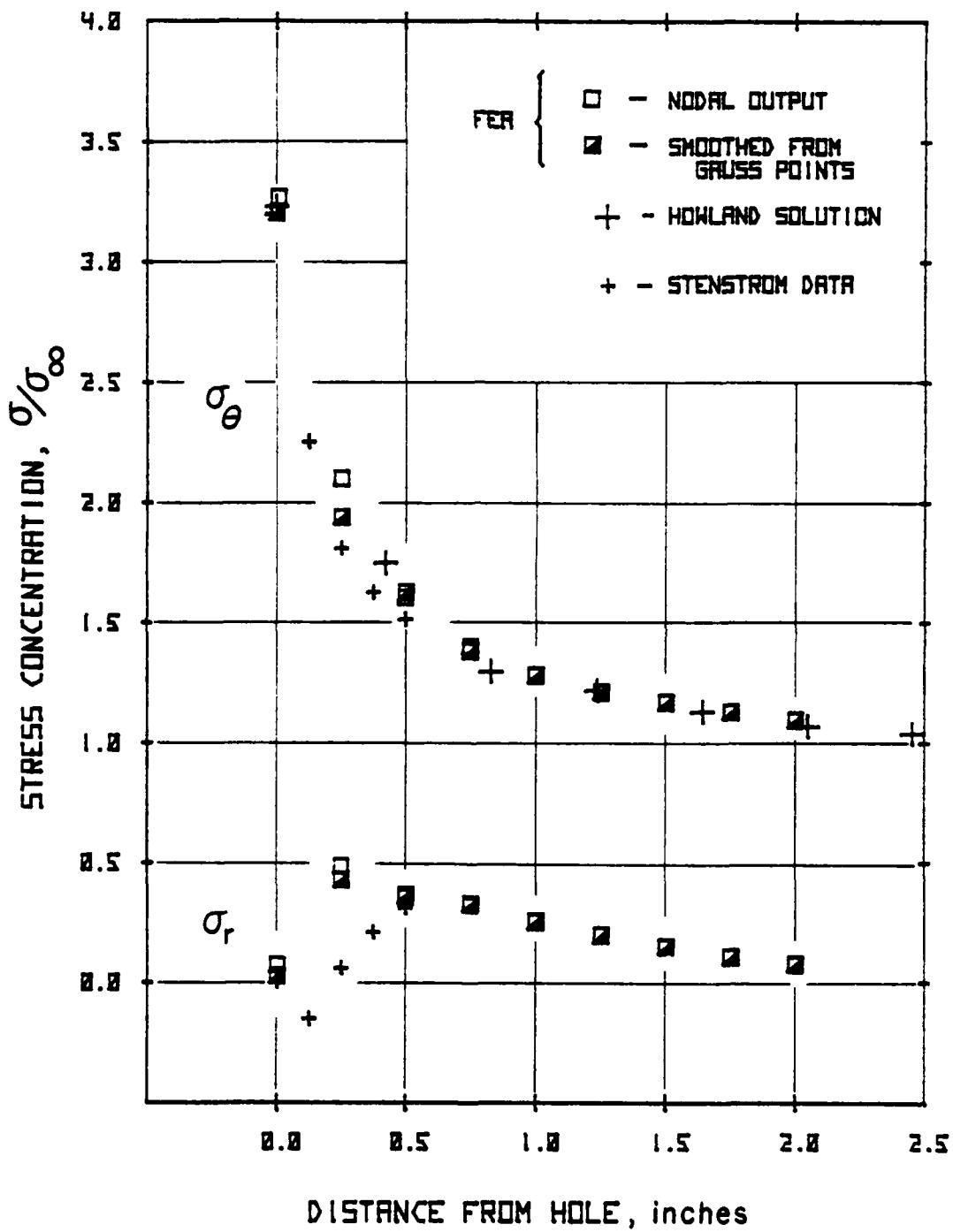


FIGURE 17

SHALLOW NOTCH LINEAR RESULTS

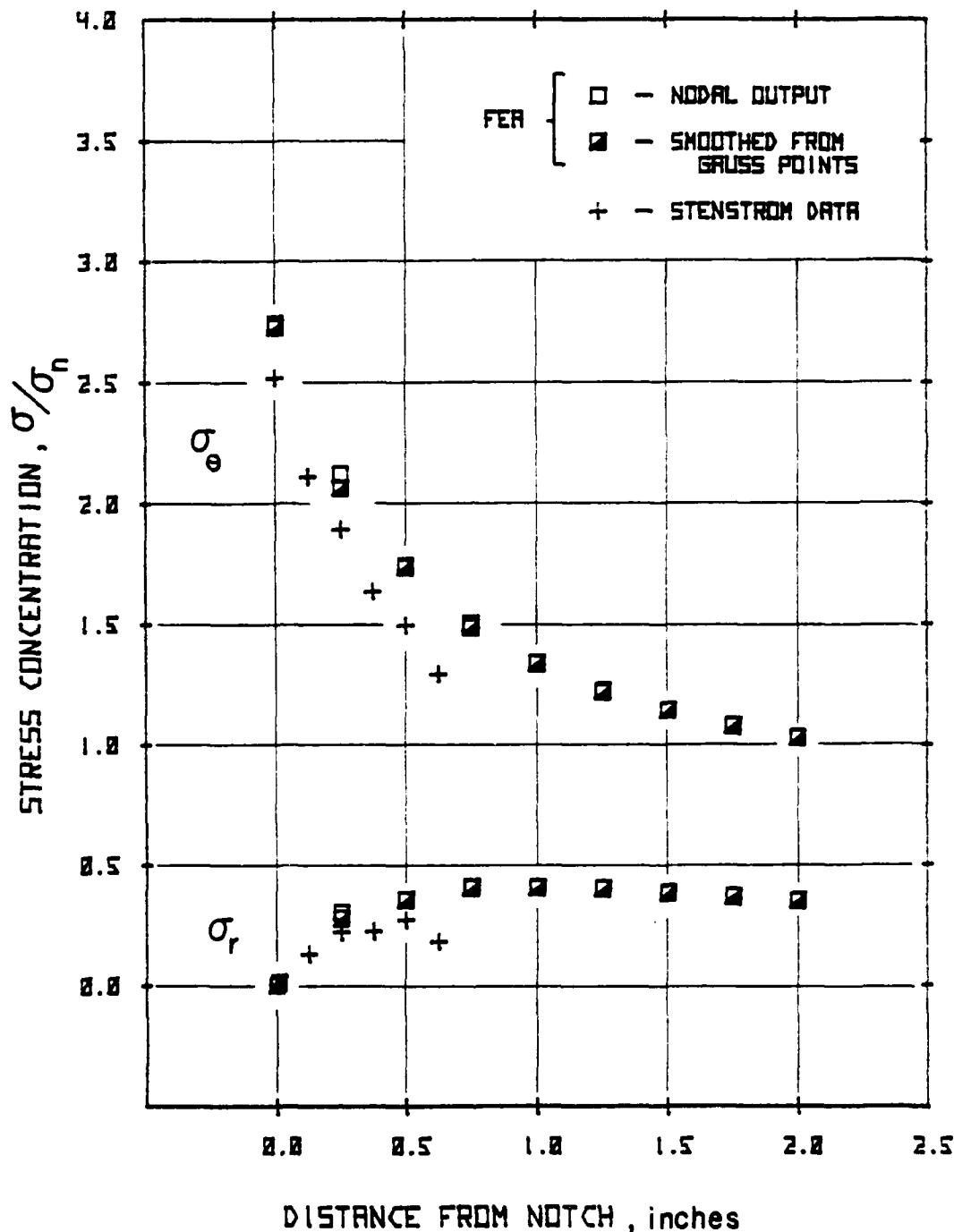


FIGURE 18

DEEP NOTCH LINEAR RESULTS

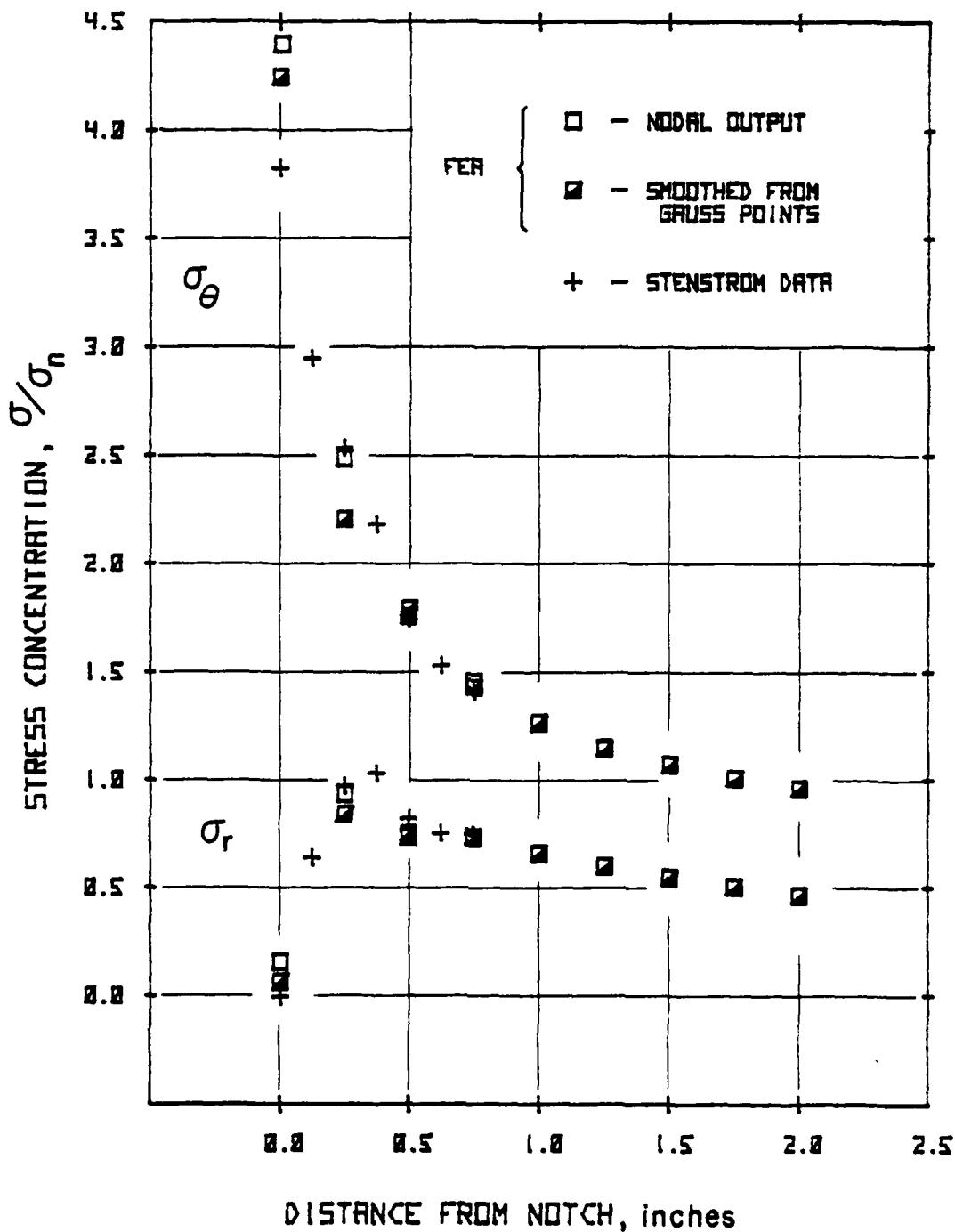


FIGURE 19
SHALLOW NOTCH SORROGUE LOAD ELASTIC-PLASTIC RESULTS

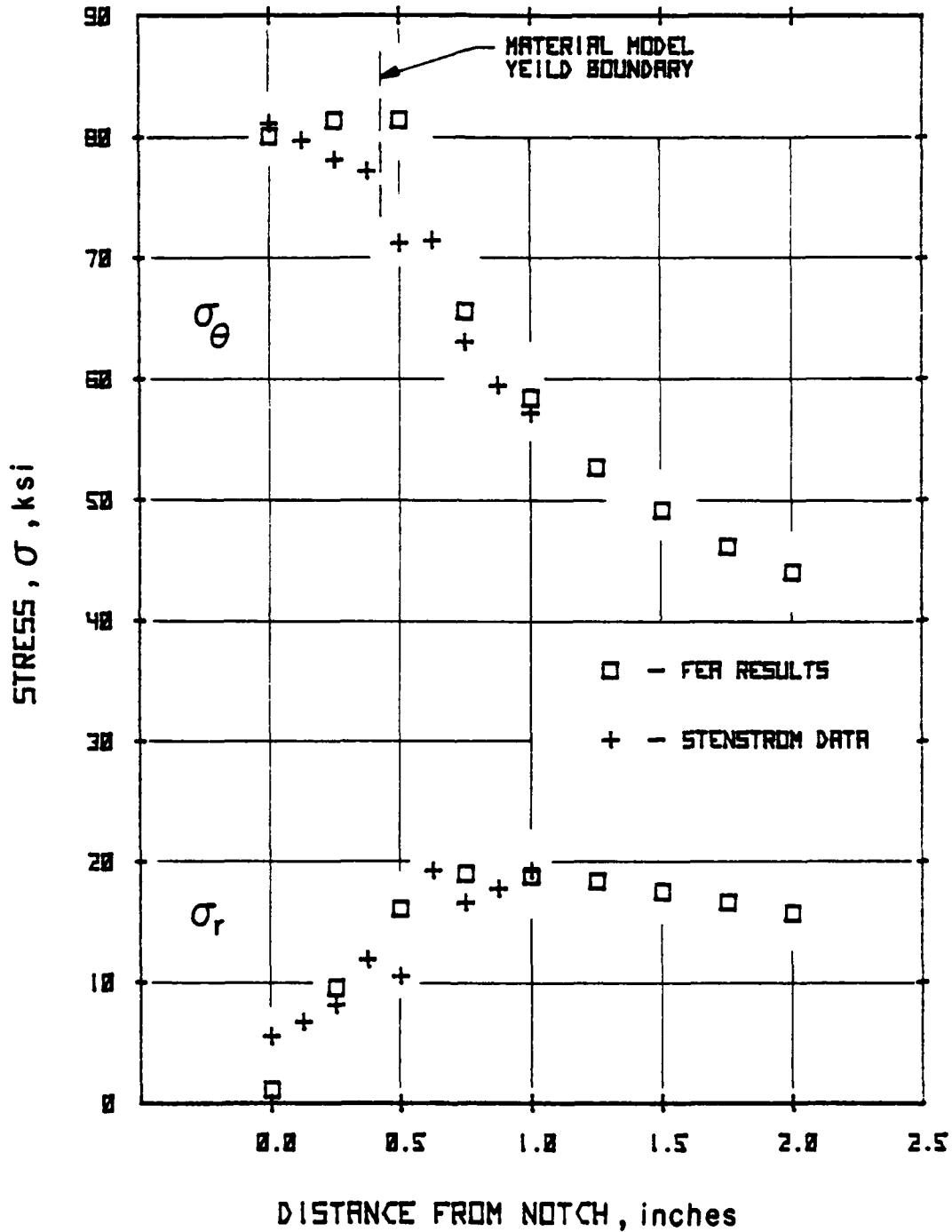


FIGURE 20
SHALLOW NOTCH 65000 LB LOAD ELASTIC-PLASTIC RESULTS

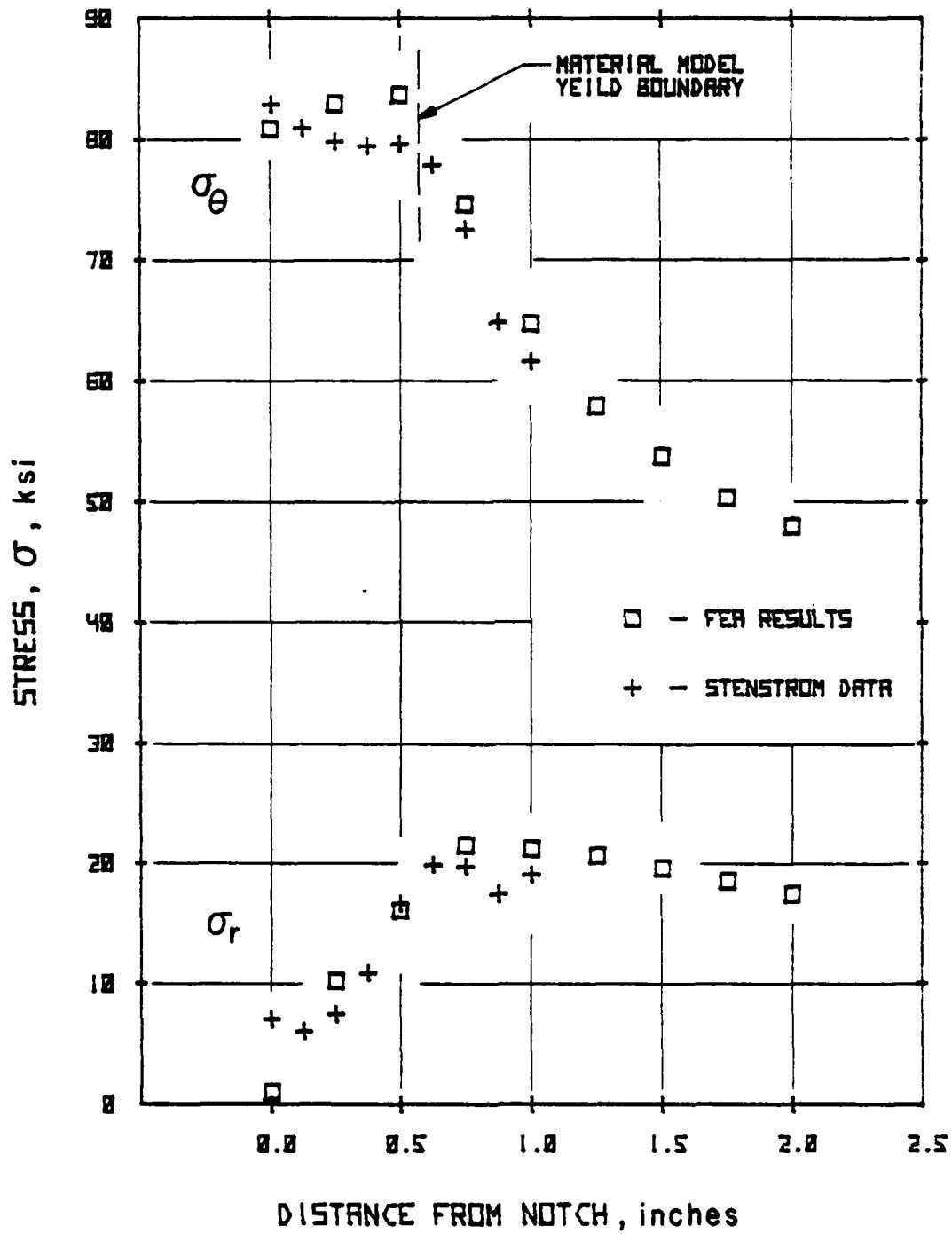
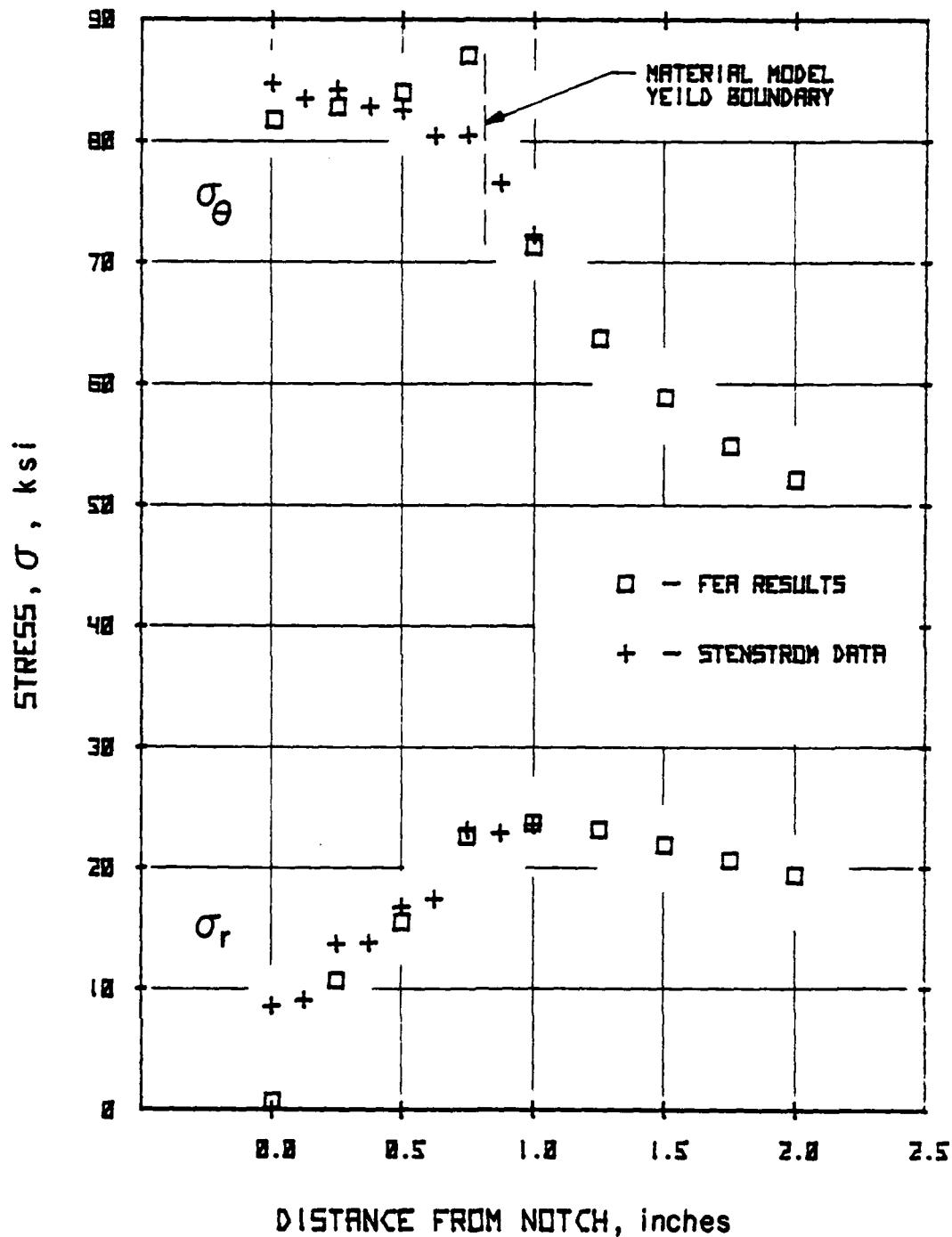


FIGURE 21

SHALLOW NOTCH 70000 LB LOAD ELASTIC-PLASTIC RESULTS



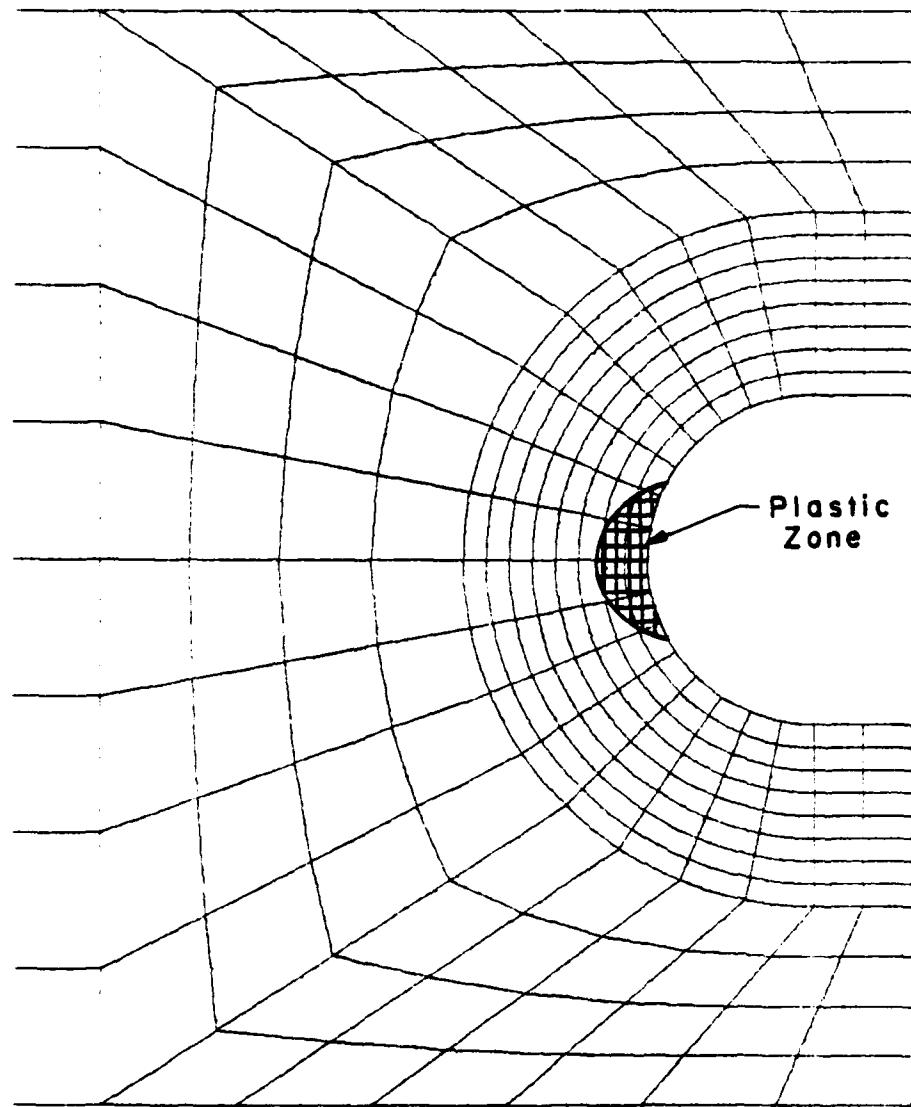


FIGURE 22

SHALLOW NOTCH 60,000 LB LOAD PLASTIC ZONE

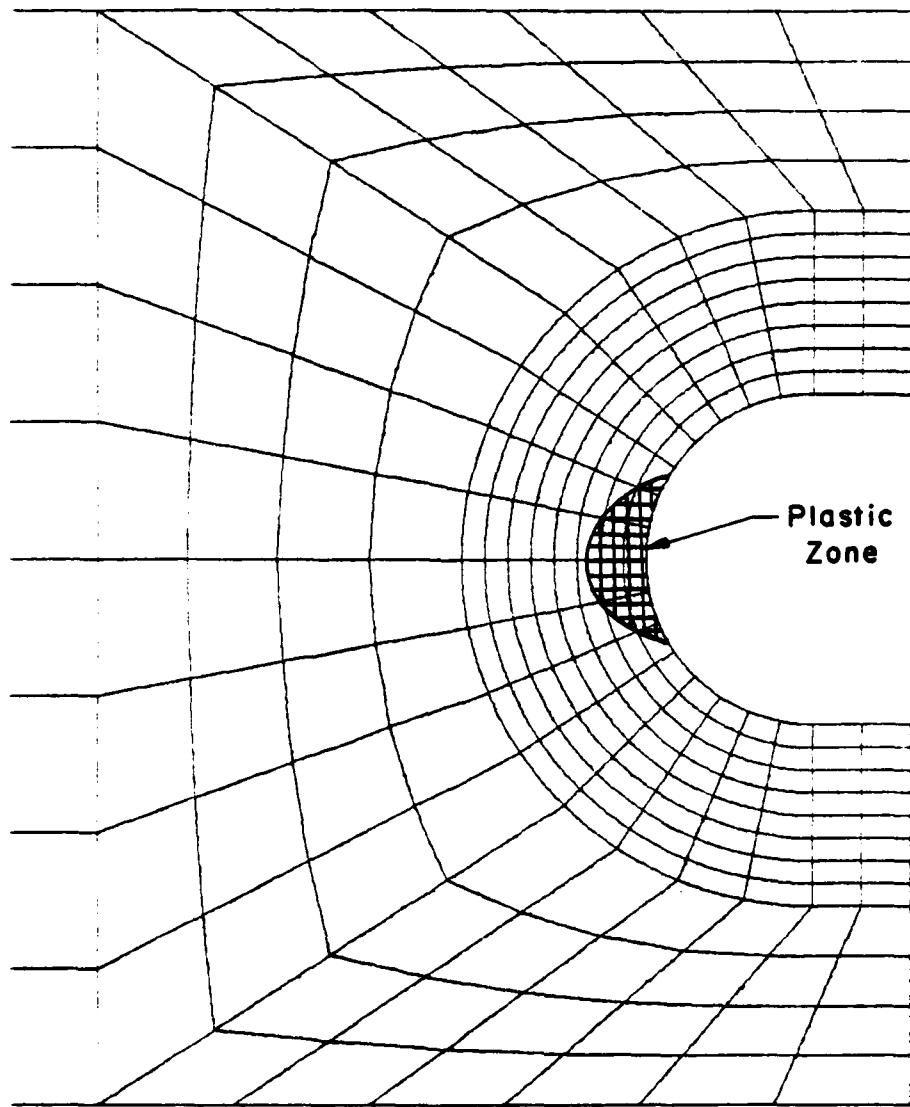


FIGURE 23

SHALLOW NOTCH 65,000 LB LOAD PLASTIC ZONE

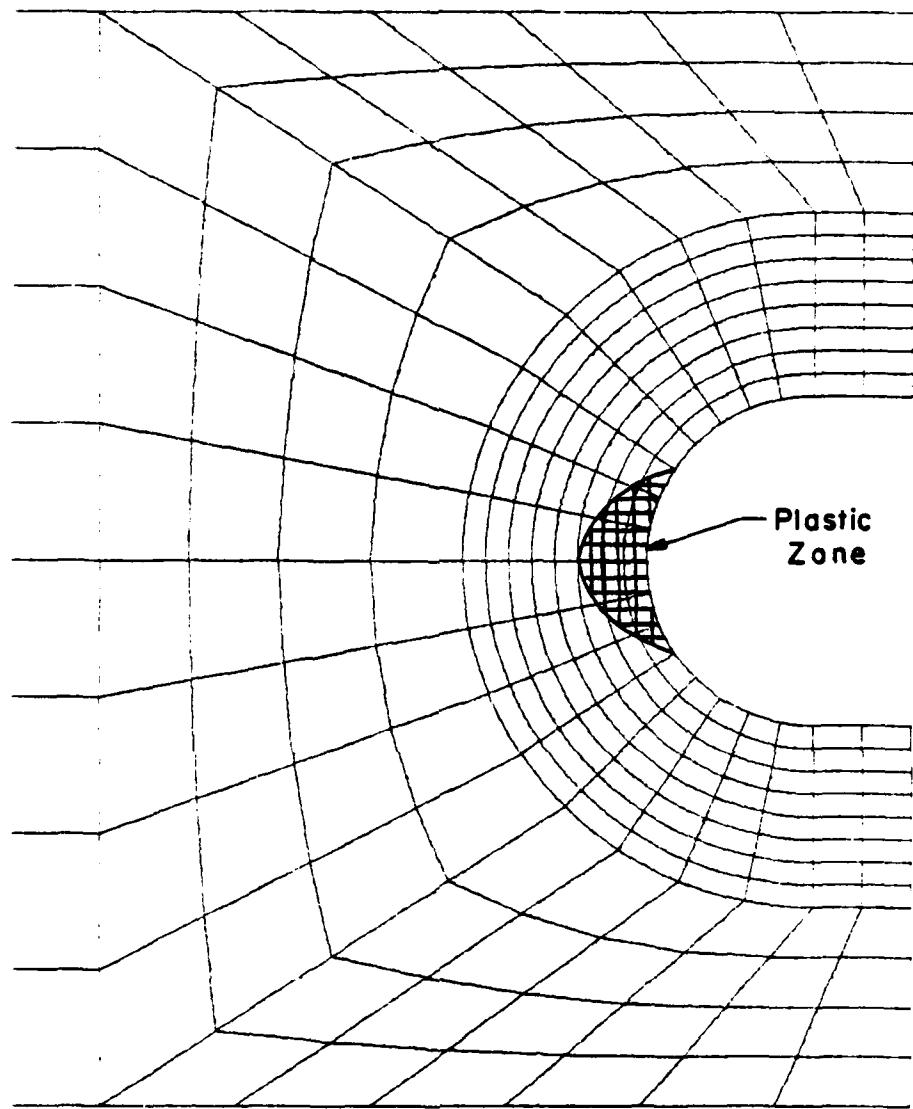


FIGURE 24

SHALLOW NOTCH 70,000 LB LOAD PLASTIC ZONE

FIGURE 25

SHALLOW NOTCH RESIDUAL σ_θ FROM 60,000 LB LOAD

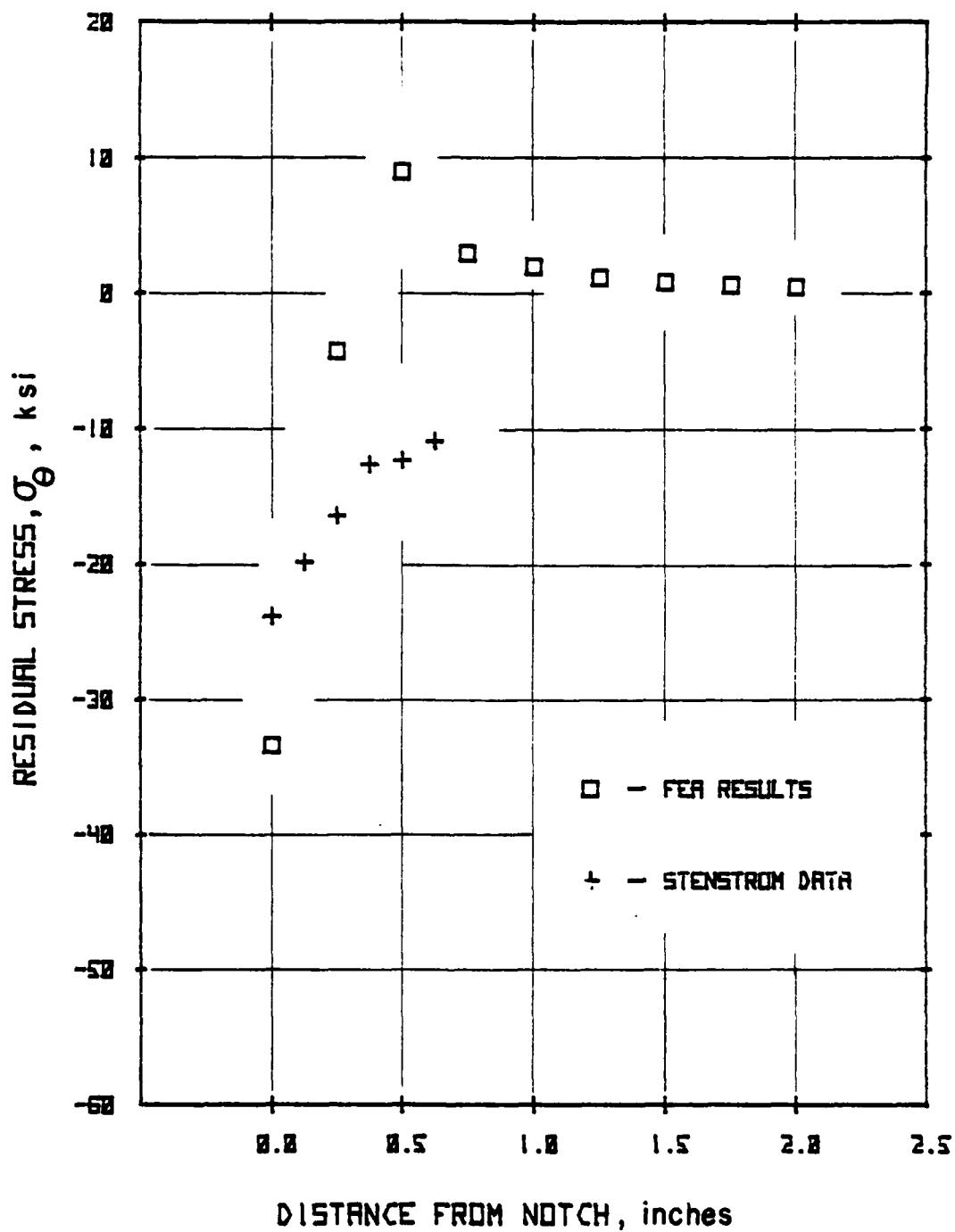


FIGURE 26

SHALLOW NOTCH RESIDUAL σ_r FROM 60,000 LB LOAD

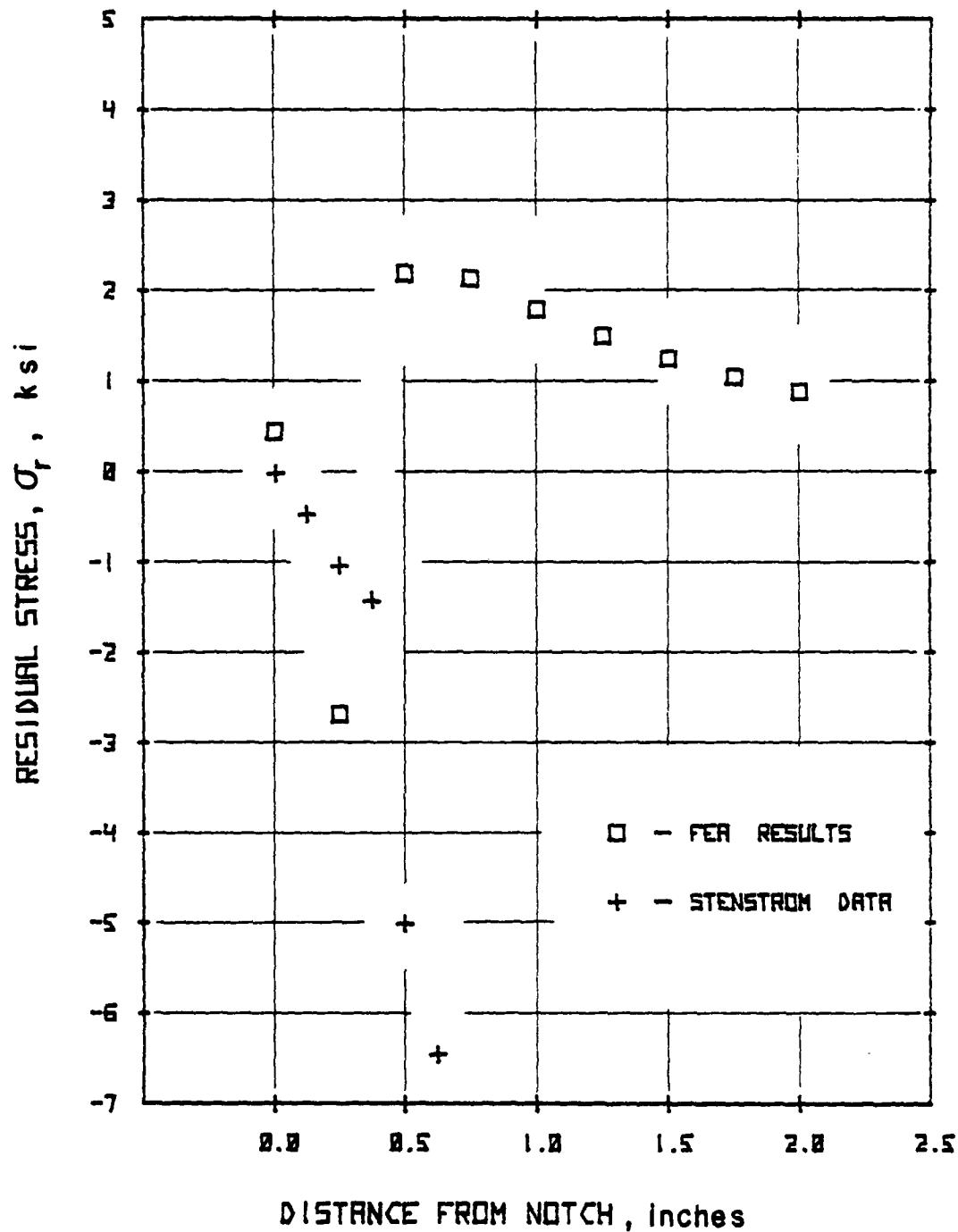


FIGURE 27

SHALLOW NOTCH RESIDUAL σ_θ FROM 65,000 LB LOAD

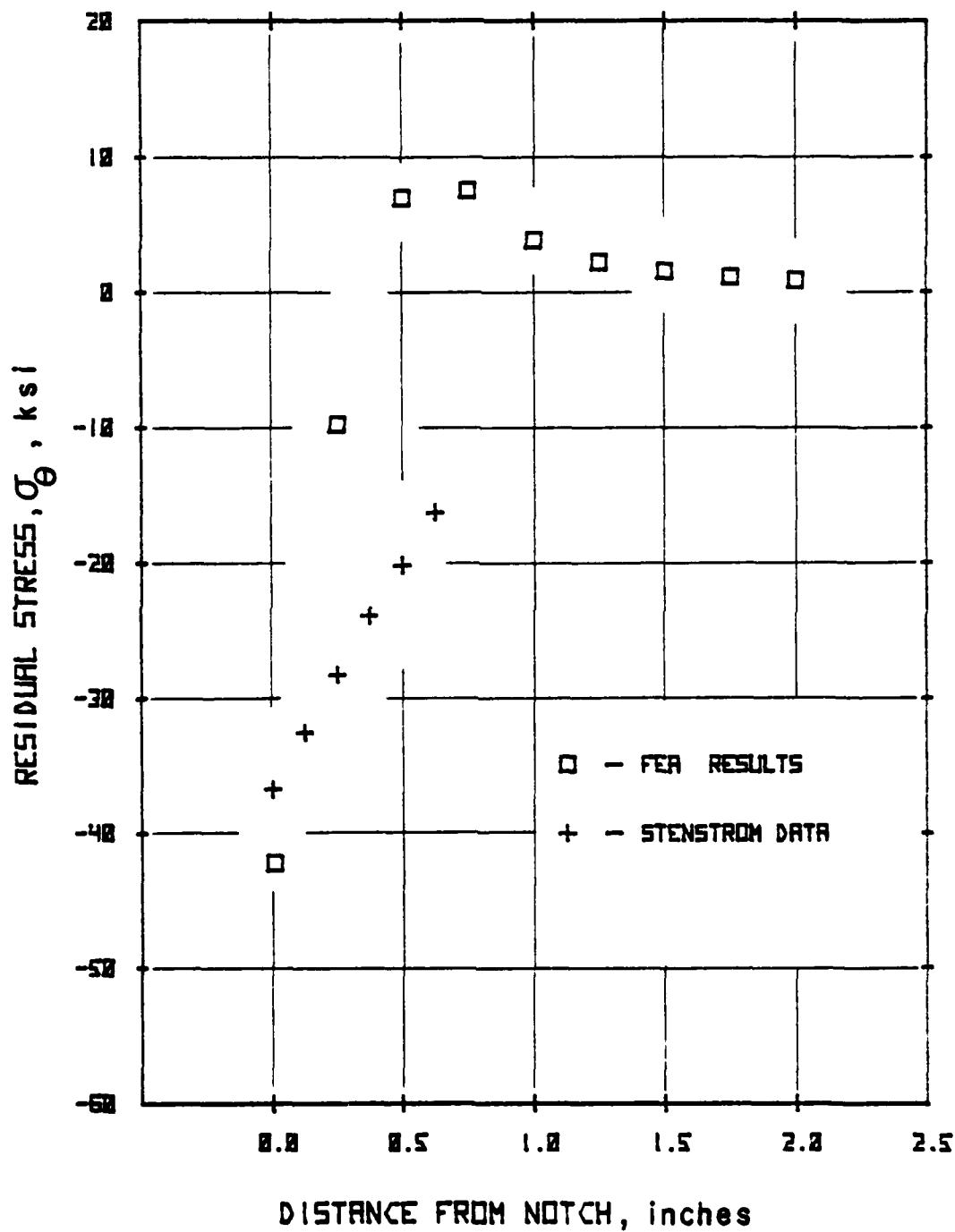


FIGURE 28

SHALLOW NOTCH RESIDUAL σ_r FROM 65,000 LB LOAD

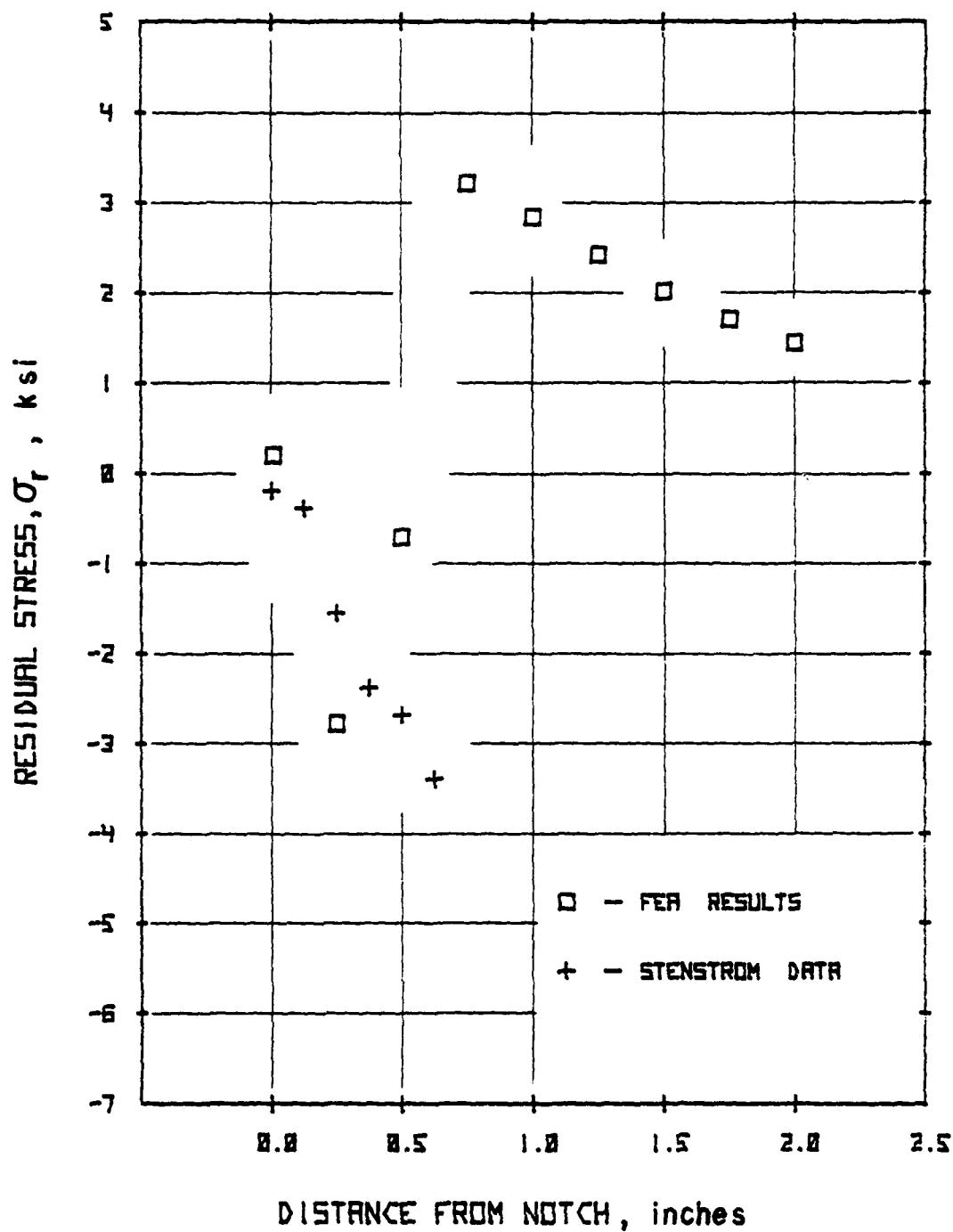


FIGURE 29

SHALLOW NOTCH RESIDUAL σ_θ FROM 70,000 LB LOAD

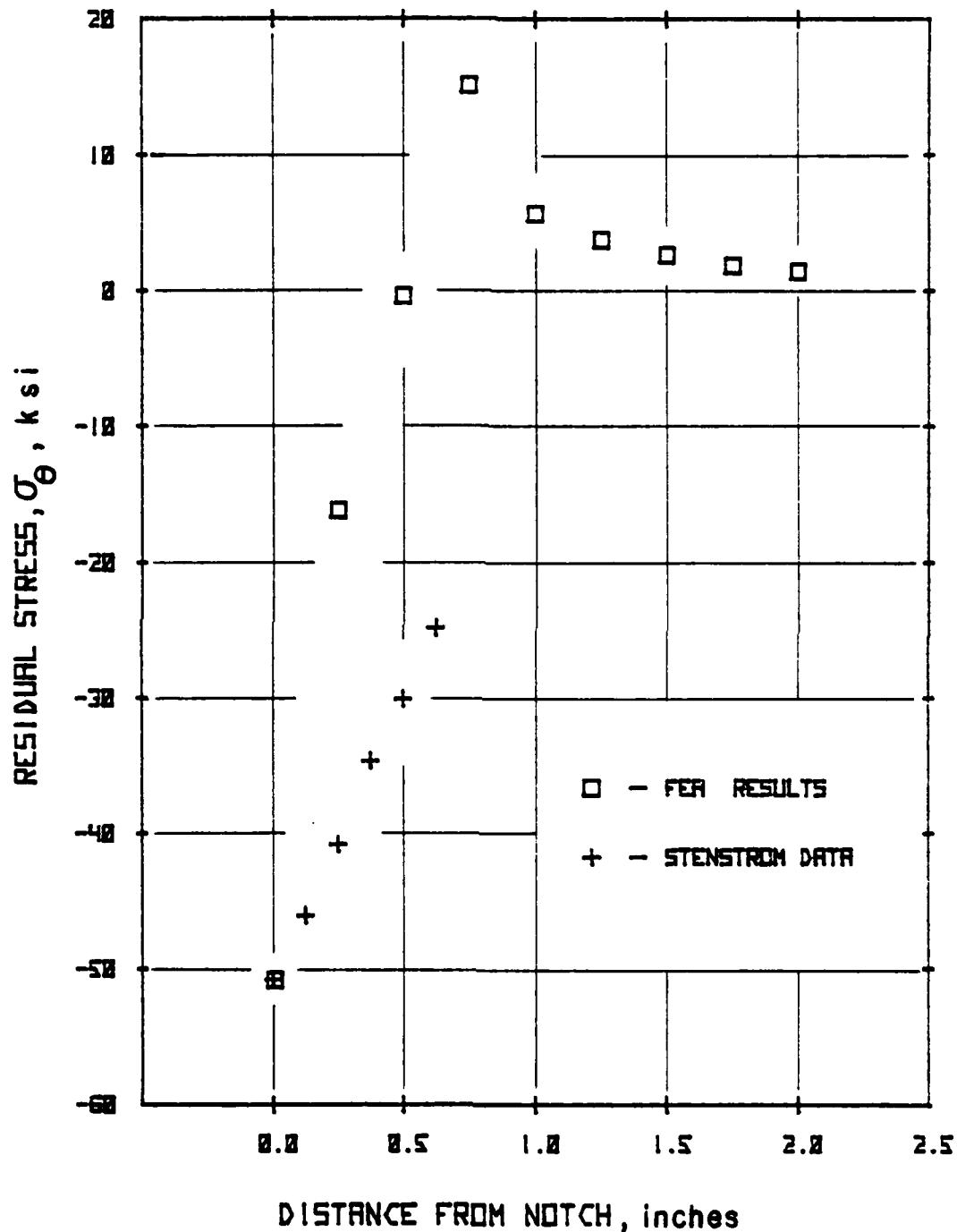


FIGURE 30

SHALLOW NOTCH RESIDUAL σ_r FROM 70,000 LB LOAD

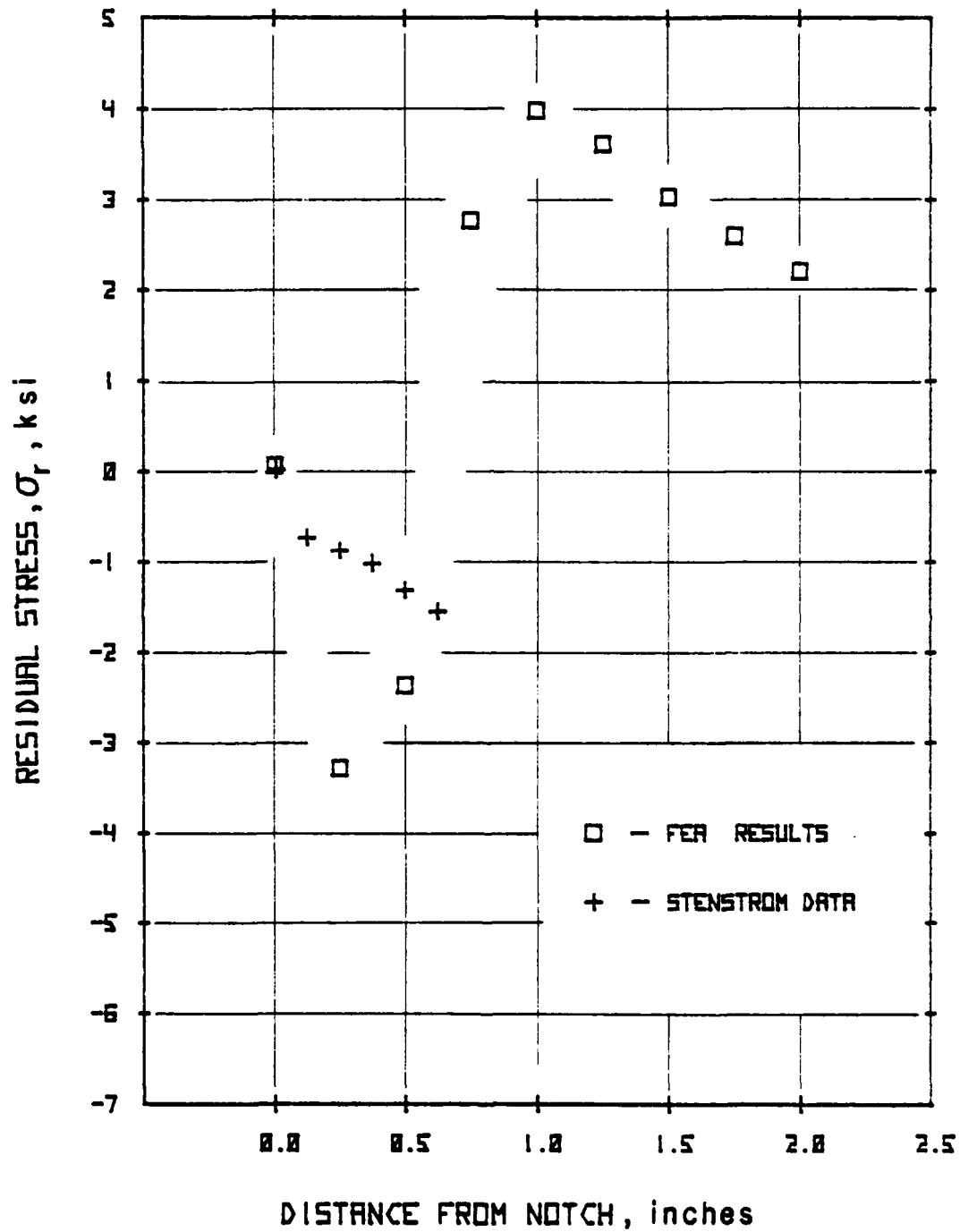


FIGURE 31
DEEP NOTCH PLASTIC LOADING RESULTS

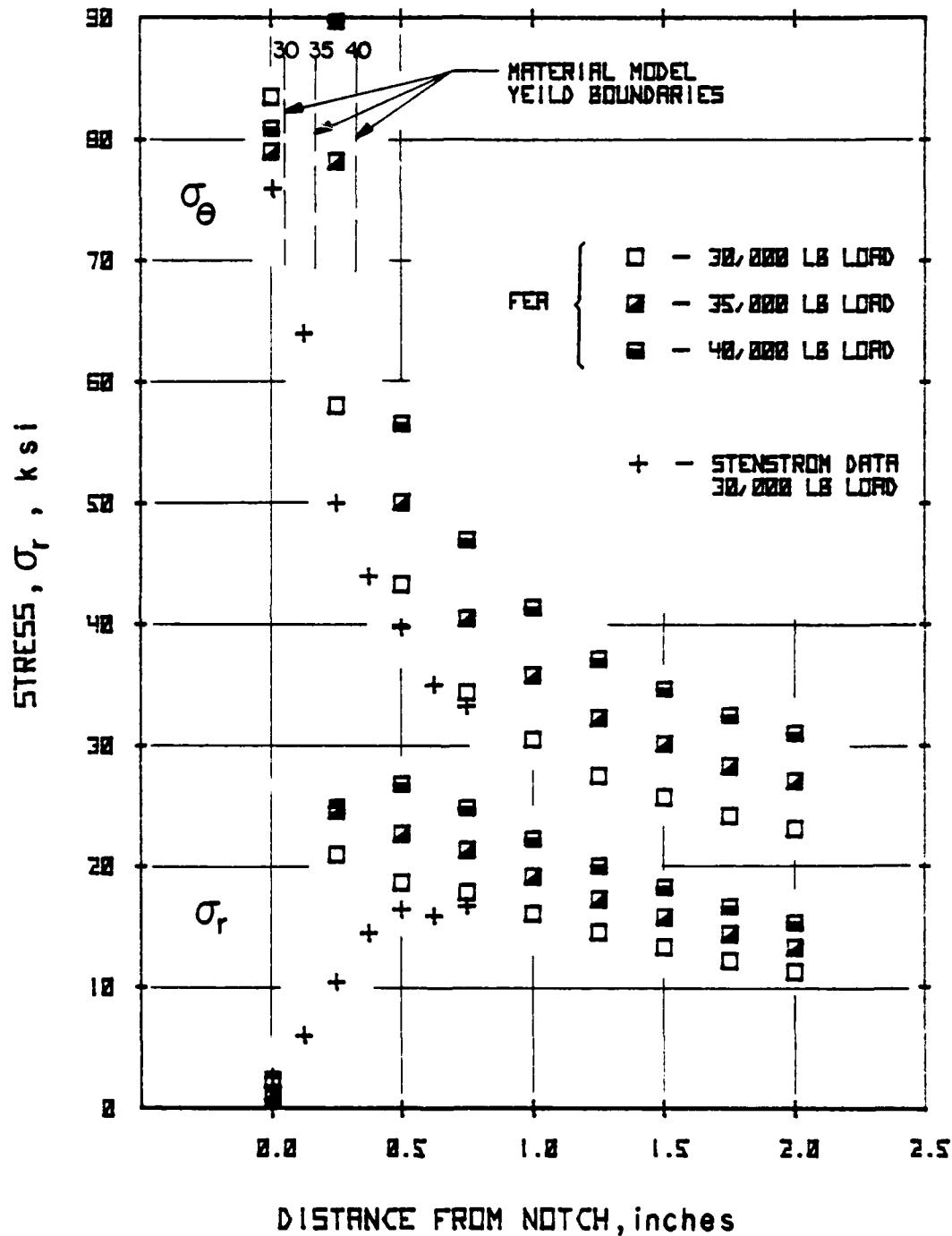


FIGURE 32

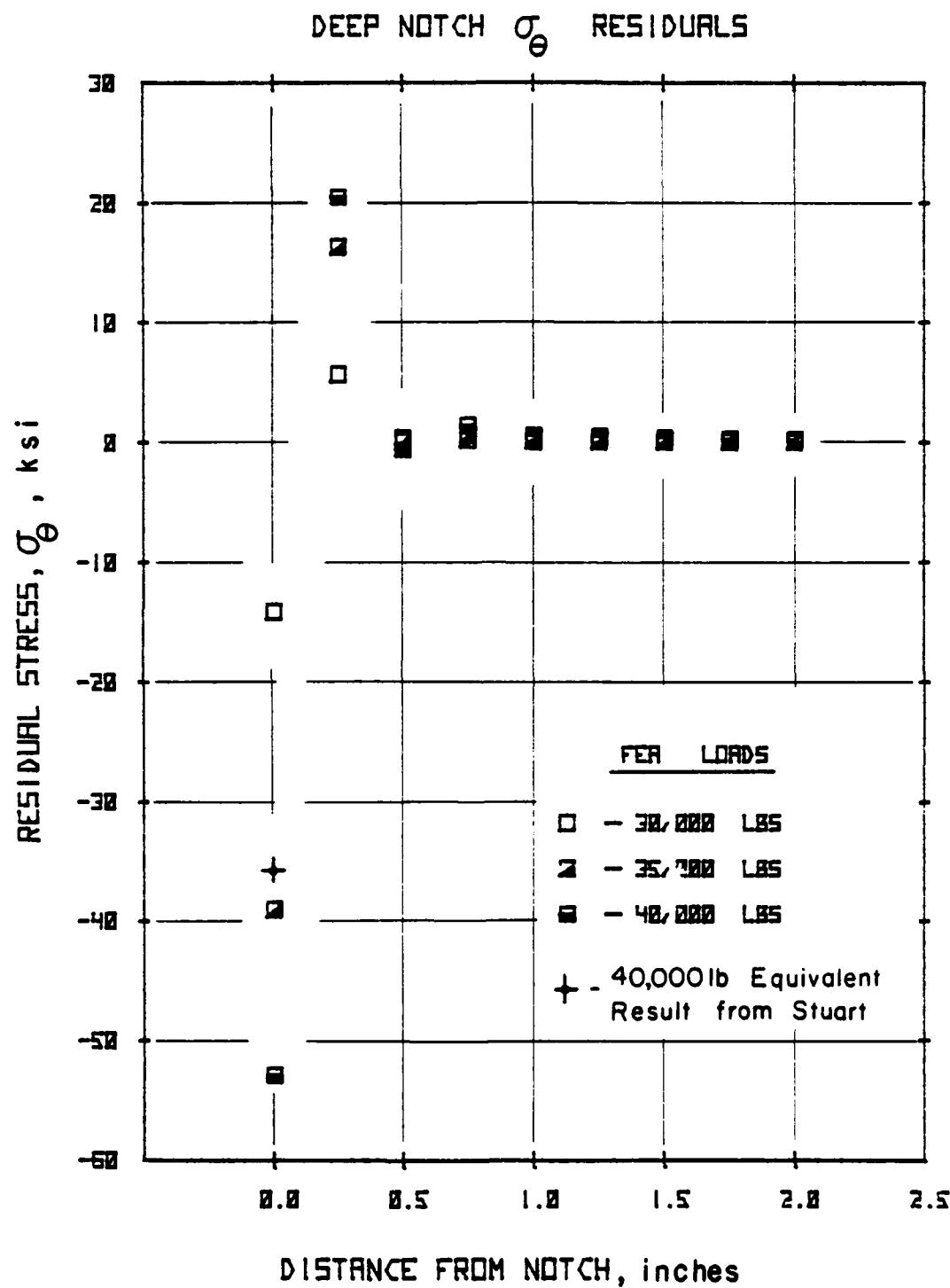
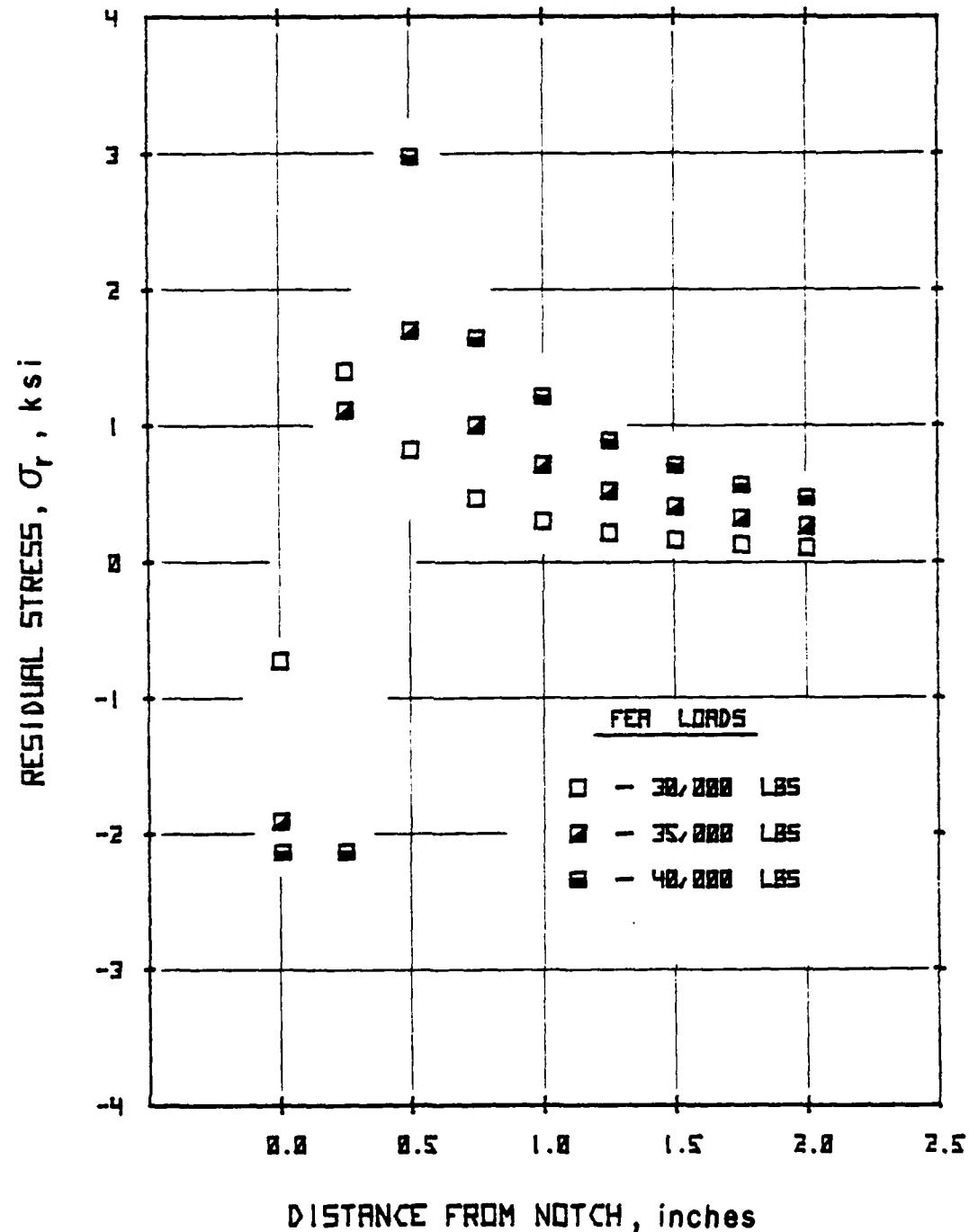


FIGURE 33
DEEP NOTCH σ_r RESIDUALS



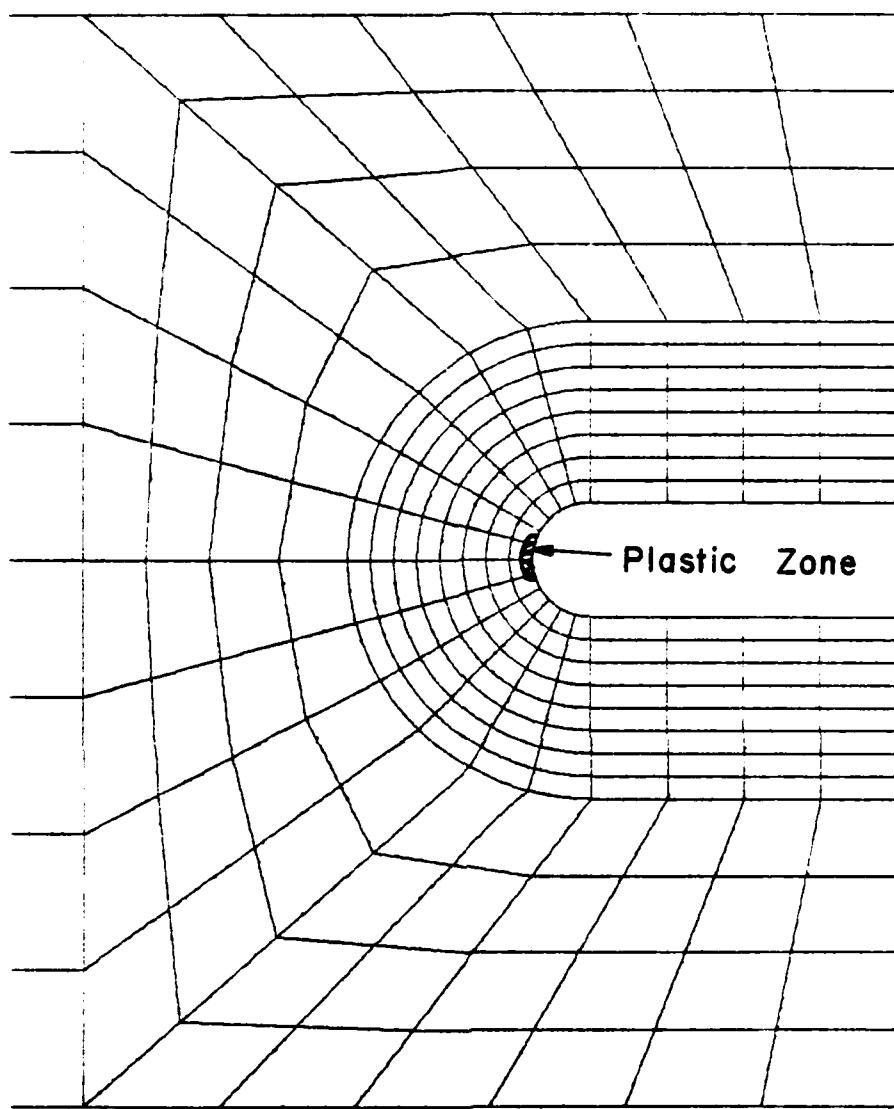


FIGURE 34

DEEP NOTCH 30,000 LB LOAD PLASTIC ZONE

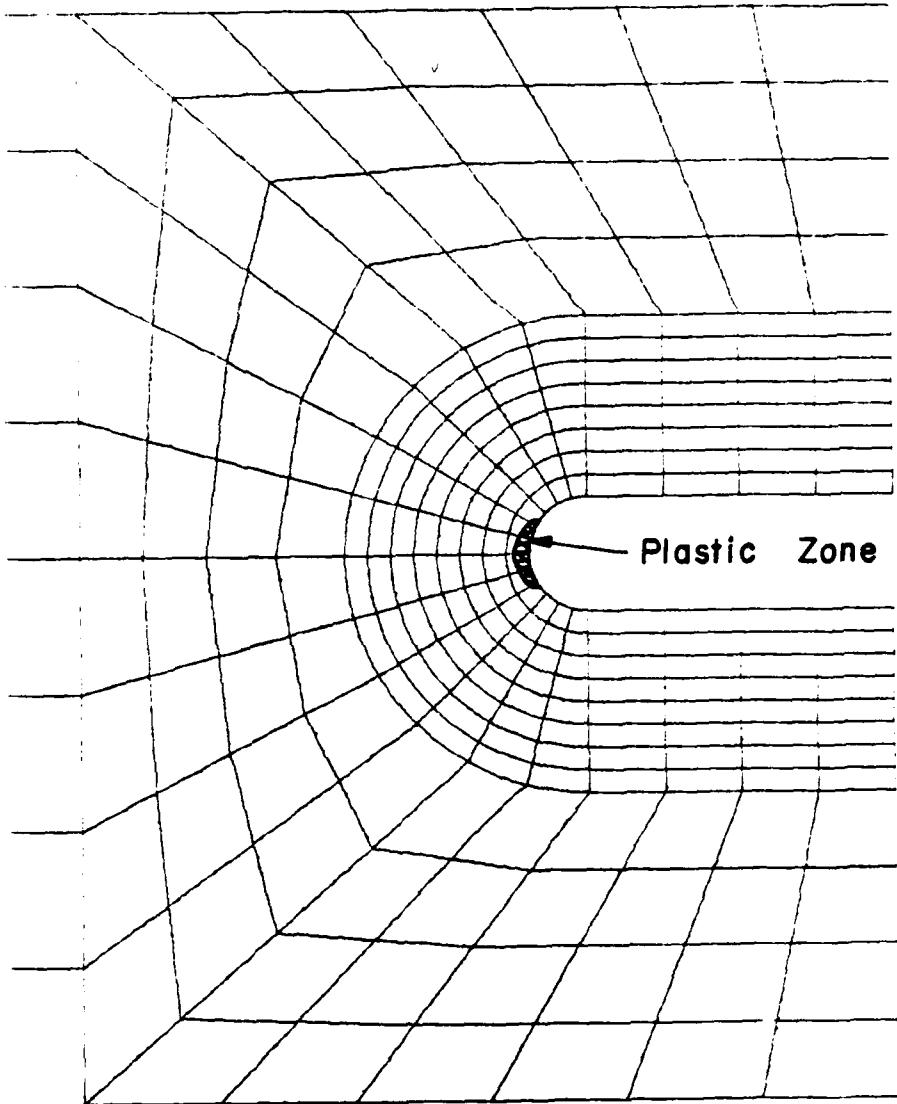


FIGURE 35

DEEP NOTCH 35,000 LB LOAD PLASTIC ZONE

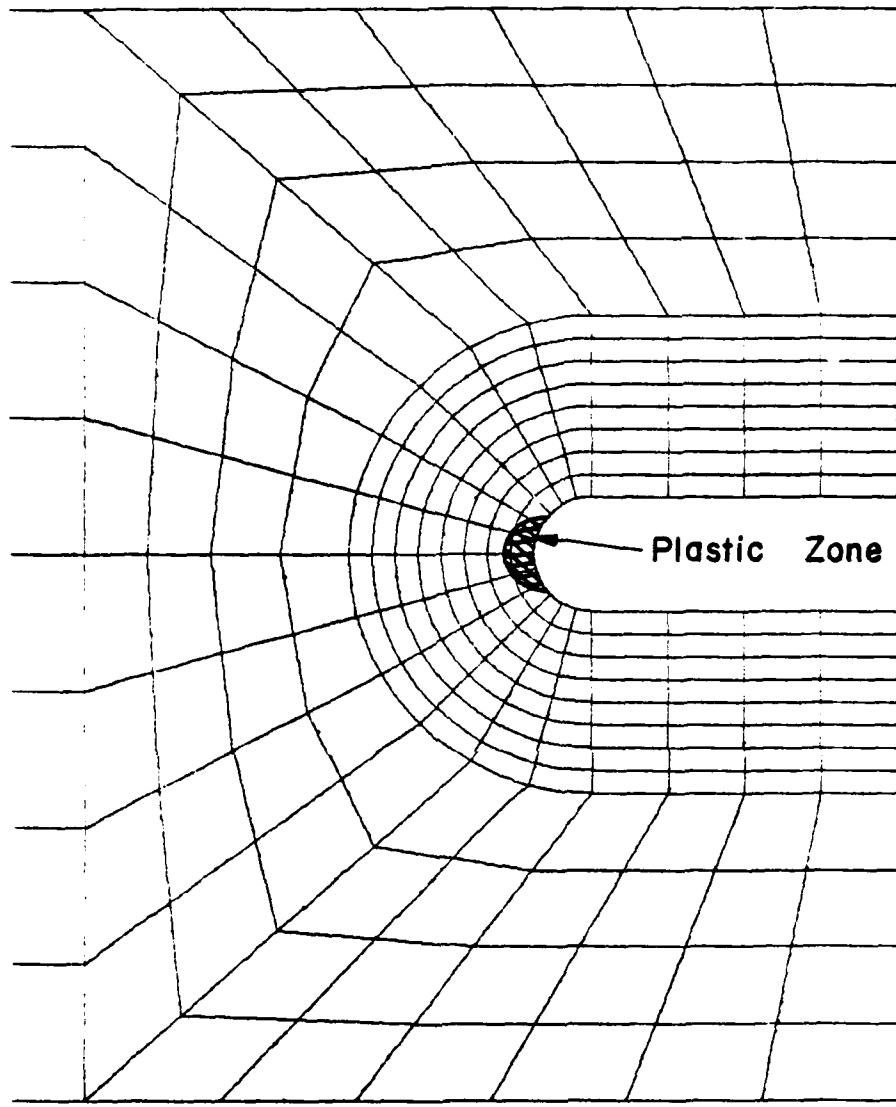
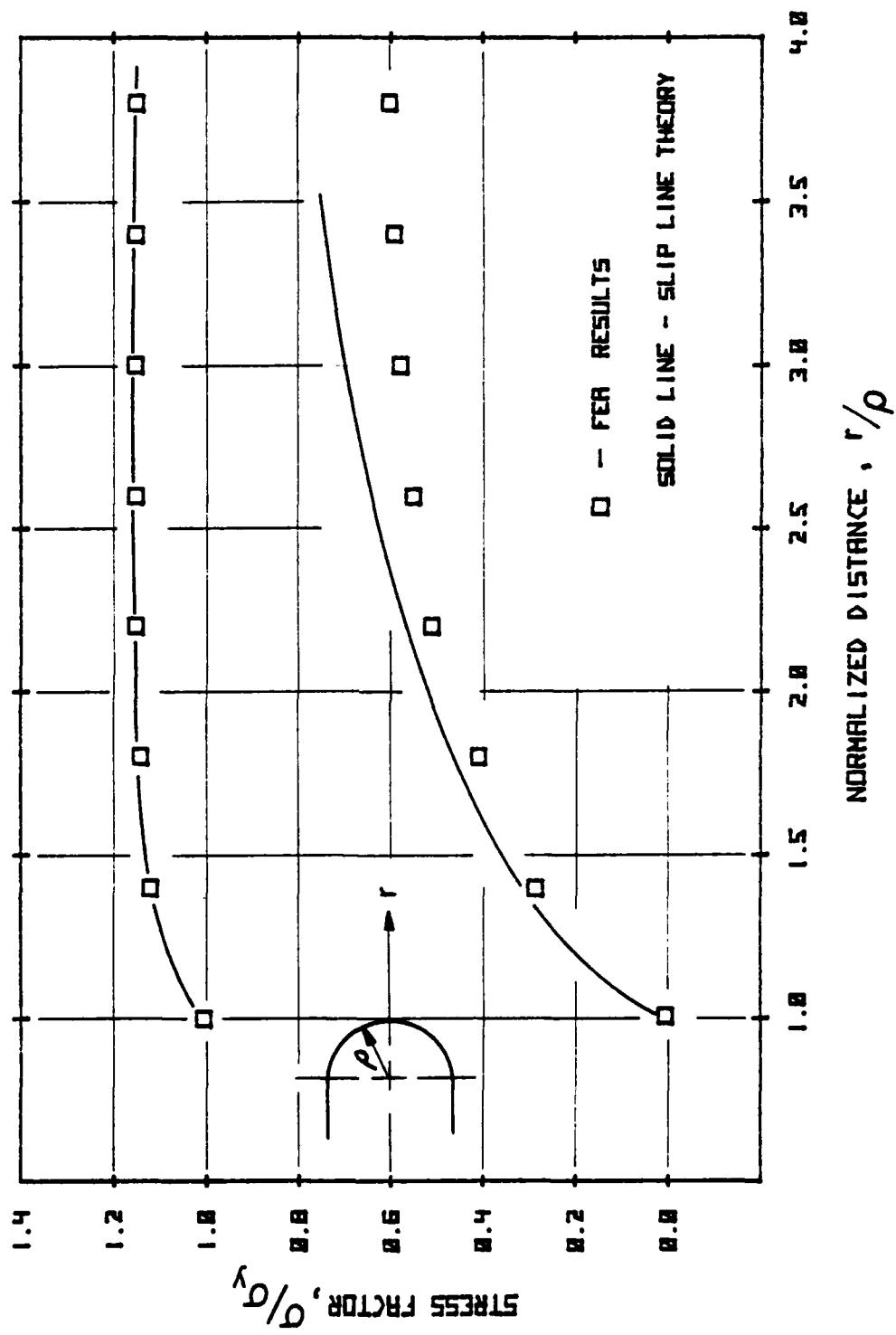


FIGURE 36

DEEP NOTCH 40,000 LB LOAD PLASTIC ZONE

FIGURE 37
RIGID-PERFECTLY-PLASTIC RESULTS



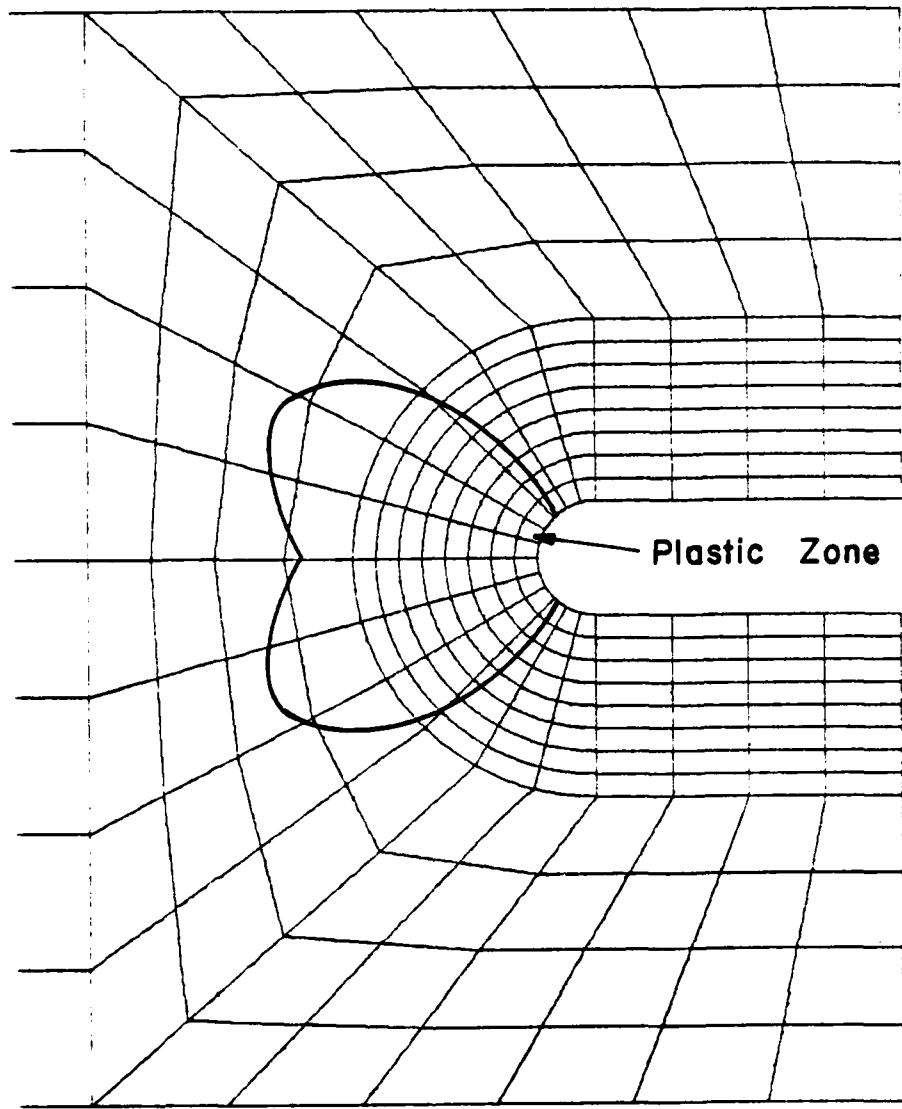


FIGURE 38

RIGID-PERFECTLY-PLASTIC INITIAL PLASTIC ZONE

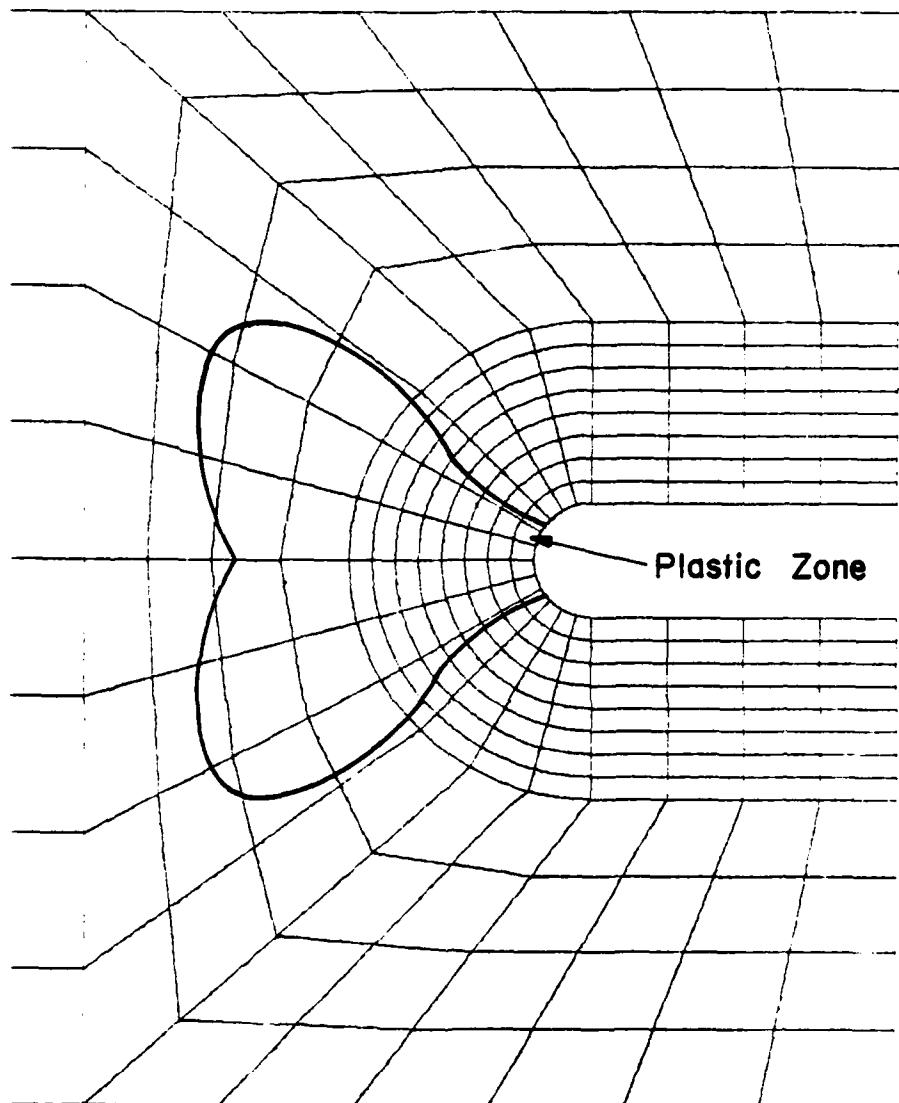


FIGURE 39

RIGID-PERFECTLY-PLASTIC INTERMEDIATE PLASTIC ZONE

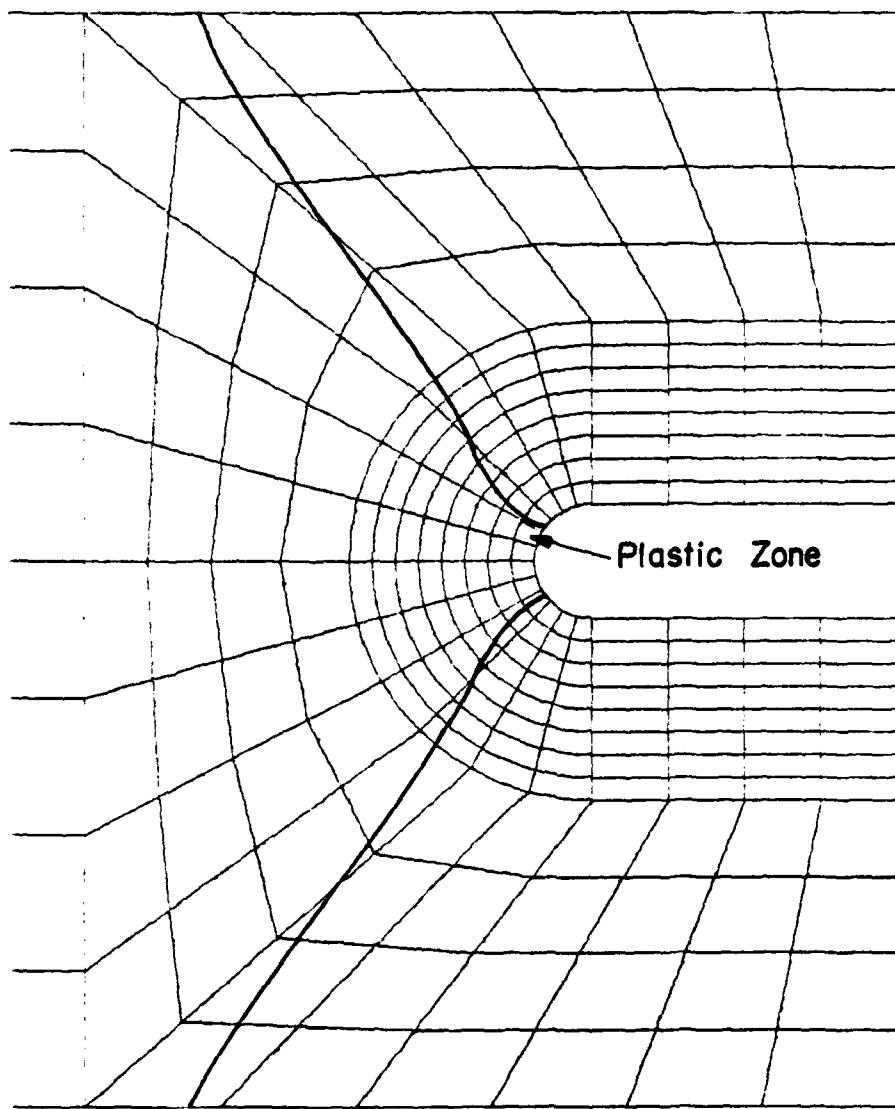


FIGURE 40

RIGID-PERFECTLY-PLASTIC FINAL PLASTIC ZONE

TABLE I. MTS AND REIHLE 5 GAGE TEST RESULTS

All Strains are 10^{-6} in/in

MTS Test Machine

LOAD lbs	STRAINS				
	ϵ_1	ϵ_2	ϵ_3	ϵ_4	ϵ_5
1000	-3	421	814	1235	1638
2000	338	1003	1619	2279	2903
3000	960	1714	2431	3186	3908

REIHLE Test Machine

LOAD lbs	STRAINS				
	ϵ_1	ϵ_2	ϵ_3	ϵ_4	ϵ_5
1000	800	763	755	741	726
2000	1564	1540	1546	1543	1535
3000	2370	2334	2363	2377	2388

TABLE II. MTS SPECIMEN A TEST RESULTS

Load, lbs.	Cross-section = 0.03975 in ²	Strain, $\epsilon, 10^{-6}$ in/in
256		615
503		1304
750		1801
1005		2423
1255		3042
1505		3665
1778		4352
2003		4925
2252		5587
2508		6355
2755		7365
2905		8230
2984		9045
3037		10150

TABLE III. MTS SPECIMEN B TEST RESULTS

Load, lbs.	Cross-section = 0.03975 in ²	Strain, ϵ , 10^{-6} in/in
	500	
750		1860
1000		2450
1250		3060
1500		3700
1750		4320
2000		4950
2250		5630
2500		6320
2750		7040
2900		7800
3000		8650
3050		10200
3100		11550
3125		12800

TABLE IV. MTS SPECIMEN C TEST RESULTS

Cross-section = 0.03975 in²

Load, lbs.	Strain, $\epsilon, 10^{-6}$ in/in
272	663
500	1420
762	1950
1000	2451
1231	3005
1506	3630
1755	4315
2000	4940
2255	5687
2500	6325
2750	7030
2900	8120
3000	9200
3060	10500
3100	11500
3125	12500

TABLE V. REINHLE SPECIMEN TEST RESULTS

Cross-section = 0.12 in²

LOAD lbs.	STRAIN, ϵ_1 10^{-6} in/in	STRAIN, ϵ_2 10^{-6} in/in
500	345	-115
1000	720	-140
1500	1114	-184
2000	1501	-242
2500	1895	-314
3000	2291	-395
3500	2795	-471
4000	3295	-550
4500	3795	-630
5000	4295	-710
5500	4795	-794
6000	5295	-874
6500	5795	-957
7000	6295	-1044
7500	6795	-1133
8000	7295	-1223
8500	7795	-1315
9000	8295	-1408
9500	12245	-4435

TABLE VI.

 $\lambda = 0.2$ HOWLAND DATA

DISTANCE FROM HOLE in.	$\sigma_\theta / \sigma_\infty$
0.0	3.14
0.5	1.57
1.0	1.26
1.5	1.16
2.0	1.11
2.5	1.07
3.0	1.05
3.5	1.01
4.0	0.97

TABLE VII.

 $\lambda = 0.25$ HOWLAND DATA

DISTANCE FROM HOLE in.	$\sigma_\theta / \sigma_\infty$
0.000	3.23
0.422	1.75
0.828	1.30
1.234	1.22
1.641	1.13
2.047	1.07
2.453	1.04
2.859	0.97
3.063	0.95

TABLE VIII. $\lambda = 0.2$ FEA RESULTS - NODAL OUTPUT

DISTANCE FROM HOLE, in.	$\sigma_\infty = 5000$ psi	
	σ_θ , psi	σ_r , psi
0.00	15938.2	351.7
0.25	10259.7	2375.8
0.50	7875.7	1783.5
0.75	6877.0	1673.1
1.00	6307.4	1334.7
1.25	5995.4	1087.2
1.50	5797.7	874.4
1.75	5638.9	700.5
2.00	5509.8	556.8

TABLE IX. $\lambda = 0.2$ FEA RESULTS - GAUSS OUTPUT

DISTANCE FROM HOLE, in.	$\sigma_\infty = 5000$ psi	
	σ_θ , psi	σ_r , psi
0.00	15595.4	126.9
0.25	9482.6	1104.1
0.50	7953.8	1326.6
0.75	6783.7	1650.4
1.00	6316.1	1348.7
1.25	5975.6	1092.8
1.50	5795.8	880.1
1.75	5623.6	645.7
2.00	5499.9	554.9

TABLE X. $\lambda = 0.25$ FEA RESULTS - NODAL OUTPUT

DISTANCE FROM HOLE, in.	$\sigma_\infty = 4500$ psi	
	σ_θ , psi	σ_r , psi
0.00	14745.5	329.8
0.25	9447.0	1181.1
0.50	7232.8	1599.3
0.75	6304.7	1473.5
1.00	5771.7	1139.6
1.25	5471.2	891.6
1.50	5266.9	671.0
1.75	5091.7	489.1
2.00	4940.2	354.5

TABLE XI. $\lambda = 0.25$ FEA RESULTS - GAUSS OUTPUT

DISTANCE FROM HOLE, in.	$\sigma_\infty = 4500$ psi	
	σ_θ , psi	σ_r , psi
0.00	14422.8	121.2
0.25	8721.2	1927.1
0.50	7317.1	1639.7
0.75	6218.6	1452.4
1.00	5779.6	1151.5
1.25	5453.3	891.0
1.50	5267.1	677.3
1.75	5085.9	486.9
2.00	4937.9	343.8

TABLE XII. SHALLOW NOTCH FEA LINEAR RESULTS - NODAL

$$\sigma_n = 4225 \text{ psi}$$

DISTANCE FROM NOTCH, in.	σ_θ , psi	σ_r , psi
0.00	11584.9	94.5
0.25	8966.1	1279.9
0.50	7335.6	1591.3
0.75	6347.3	1731.7
1.00	5655.0	1713.2
1.25	5184.1	1703.5
1.50	4832.4	1626.7
1.75	4564.7	1560.6
2.00	4361.1	1484.7

TABLE XIII. SHALLOW NOTCH FEA LINEAR RESULTS - GAUSS

$$\sigma_n = 4225 \text{ psi}$$

DISTANCE FROM NOTCH, in.	σ_θ , psi	σ_r , psi
0.00	11530.2	11.5
0.25	8727.1	1177.7
0.50	7355.5	1584.8
0.75	6280.9	1713.6
1.00	5661.2	1720.4
1.25	5159.3	1699.7
1.50	4834.6	1634.2
1.75	4552.8	1561.7
2.00	4353.6	1486.0

TABLE XIV. DEEP NOTCH FEA LINEAR RESULTS - NODAL

$$\sigma_n = 4800 \text{ psi}$$

DISTANCE

FROM NOTCH, in.	σ_θ , psi	σ_r , psi
0.00	21142.8	755.2
0.25	11929.9	4464.3
0.50	8436.8	3530.4
0.75	6976.6	3324.4
1.00	6076.1	3152.2
1.25	5527.0	2993.4
1.50	5139.2	2826.4
1.75	4842.4	2412.0
2.00	4617.8	2234.2

TABLE XV. DEEP NOTCH FEA LINEAR RESULTS - GAUSS

$$\sigma_n = 4800 \text{ psi}$$

DISTANCE

FROM NOTCH, in.	σ_θ , psi	σ_r , psi
0.00	20368.1	314.9
0.25	10582.5	4035.2
0.50	8606.0	3611.8
0.75	6852.2	3581.9
1.00	6086.7	3173.2
1.25	5496.1	2884.7
1.50	5141.8	2643.0
1.75	4830.1	2421.2
2.00	4632.0	2238.8

TABLE XVI. SHALLOW NOTCH FEA NONLINEAR 60,000 lb LOAD

DISTANCE FROM NOTCH,in.	NO LOAD RESIDUALS			
	σ_{θ} ,psi	σ_r ,psi	σ_{θ} ,psi	σ_r ,psi
0.00	60024.7	1094.5	-33411.4	439.1
0.25	81338.7	9523.3	-4259.0	-2690.1
0.50	81383.6	16052.6	9006.6	2188.8
0.75	65548.3	18993.2	2946.6	2133.4
1.00	58431.4	18799.6	1992.8	1796.7
1.25	52670.1	18347.9	1167.0	1501.6
1.50	49111.1	17477.1	825.7	1250.8
1.75	46080.7	16567.3	594.4	1046.7
2.00	43973.8	15667.8	471.6	885.0

TABLE XVII. SHALLOW NOTCH FEA NONLINEAR 65,000 lb LOAD

DISTANCE FROM NOTCH,in.	NO LOAD RESIDUALS			
	σ_{θ} ,psi	σ_r ,psi	σ_{θ} ,psi	σ_r ,psi
0.00	80890.4	1004.6	-42092.1	224.7
0.25	82939.3	10229.5	-9796.3	-3782.0
0.50	83642.7	16054.6	6944.3	-711.2
0.75	74622.0	21475.5	7507.7	3214.2
1.00	64770.4	21171.3	3820.9	2838.7
1.25	57891.2	20593.6	2202.8	2419.0
1.50	53766.7	19537.0	1524.5	3018.5
1.75	50321.1	18474.6	1089.5	1710.1
2.00	47946.2	17421.4	853.7	1448.0

TABLE XVIII. SHALLOW NOTCH FEA NONLINEAR 70,000lb LOAD

DISTANCE FROM NOTCH, in.	NO LOAD RESIDUALS			
	σ_θ , psi	σ_r , psi	σ_θ , psi	σ_r , psi
0.00	81957.9	704.5	-50670.6	63.9
0.25	82822.6	10668.8	-16176.6	-3285.1
0.50	84007.3	15514.4	-376.9	-2366.8
0.75	87017.4	22662.9	15132.6	3762.3
1.00	71318.0	23765.2	5681.1	3983.0
1.25	63762.9	23215.2	3763.1	3616.6
1.50	58941.5	21920.1	2669.9	3030.0
1.75	54897.6	20670.6	1865.4	2596.7
2.00	52177.0	19426.6	1448.2	2207.0

TABLE XIX. DEEP NOTCH FEA NONLINEAR 30,000 lb LOAD

DISTANCE FROM NOTCH, in.	NO LOAD RESIDUALS			
	σ_θ , psi	σ_r , psi	σ_θ , psi	σ_r , psi
0.00	83521.0	2332.4	-14138.0	-725.2
0.25	58638.6	20971.3	5630.1	1402.5
0.50	43309.1	18640.5	385.7	824.6
0.75	34402.6	17839.1	198.3	468.9
1.00	30522.9	16076.1	125.0	300.5
1.25	27543.7	14574.8	88.9	215.3
1.50	25757.0	13328.7	67.3	160.7
1.75	24188.8	12195.0	53.5	126.3
2.00	23141.6	11265.7	44.9	100.9

TABLE XX. DEEP NOTCH FEA NONLINEAR 35,000 lb LOAD

DISTANCE FROM NOTCH,in.	NO LOAD RESIDUALS			
	σ_{θ} ,psi	σ_r ,psi	σ_{θ} ,psi	σ_r ,psi
0.00	79030.4	1258.1	-39963.3	-1909.2
0.25	78215.7	24598.9	16348.7	1113.1
0.50	50094.6	22675.1	105.7	1699.5
0.75	46516.0	21359.5	624.1	1002.8
1.00	35843.1	19156.9	380.6	716.7
1.25	32290.3	17299.1	257.8	521.7
1.50	30172.9	15782.1	203.5	405.9
1.75	28314.8	14410.8	156.9	319.5
2.00	27079.2	13294.4	133.7	261.5

TABLE XXI. DEEP NOTCH FEA NONLINEAR 40,000 lb LOAD

DISTANCE FROM NOTCH,in.	NO LOAD RESIDUALS			
	σ_{θ} ,psi	σ_r ,psi	σ_{θ} ,psi	σ_r ,psi
0.00	89929.5	835.4	-52900.9	-2121.6
0.25	88730.7	24938.8	20563.1	-2132.3
0.50	56572.6	26815.4	-610.4	2977.0
0.75	46984.6	24878.1	1468.2	1643.1
1.00	41400.6	22286.3	610.0	1316.6
1.25	37164.5	20068.3	555.9	892.7
1.50	34690.9	18287.2	438.5	710.5
1.75	32512.6	16672.2	330.7	564.8
2.00	31078.3	15368.8	282.0	470.8

TABLE XXII.

RIGID - PERFECTLY - PLASTIC RESULTS

$$\sigma_y = 73,000 \text{ psi}$$

DISTANCE FROM NOTCH, in.	σ_θ , psi	σ_r , psi
0.00	73334.4	519.9
0.25	81780.5	26869.6
0.50	83252.6	29813.6
0.75	84074.1	37321.1
1.00	84103.1	40244.0
1.25	84091.8	42244.0
1.50	84051.5	43249.8
1.75	84005.0	44032.9
2.00	83958.1	43875.6

TABLE XXIII. EXPERIMENTAL DATA $\lambda = 0.25$ HOLE LINEAR LOADING

$$\sigma_\infty = 10,749 \text{ psi}$$

DISTANCE FROM HOLE, in.	σ_θ , psi	σ_r , psi
0.000	35221.0	49.5
0.125	24639.0	-1650.5
0.250	19809.5	669.5
0.375	17799.0	2311.5
0.500	16588.0	3432.0

TABLE XXIV. EXPERIMENTAL DATA SHALLOW NOTCH LINEAR LOADING

15,000 lb Load

DISTANCE FROM HOLE, in.	σ_{θ} , psi	σ_r , psi
0.000	26611.7	53.2
0.125	22295.0	1392.6
0.250	19999.3	2353.7
0.375	17277.5	2387.6
0.500	15798.7	2880.5
0.625	13679.1	1929.3

TABLE XXV. EXPERIMENTAL DATA SHALLOW NOTCH 60,000 lb LOAD

DISTANCE FROM NOTCH, in.	NO LOAD RESIDUALS			
	σ_{θ} , psi	σ_r , psi	σ_{θ} , psi	σ_r , psi
0.000	81111.5	5551.0	-23842.0	4.0
0.125	79710.7	6735.8	-19837.0	-475.0
0.250	78112.0	8141.5	-16409.0	-1044.0
0.375	77189.3	11899.5	-12620.0	-1426.0
0.500	71207.2	10509.4	-12319.0	-5008.0
0.625	71436.4	19260.0	-10906.0	-6453.0
0.750	63045.8	16580.9		
0.875	59477.4	17748.6		
1.000	57146.4	19275.4		

TABLE XXVI. EXPERIMENTAL DATA SHALLOW NOTCH 65,000 lb LOAD

DISTANCE FROM NOTCH,in.	σ_θ ,psi	σ_r ,psi	NO LOAD RESIDUALS	
			σ_θ ,psi	σ_r ,psi
0.000	83073.9	7053.0	-36745.0	-131.0
0.125	80957.6	6073.9	-32540.0	-337.0
0.250	79854.9	7468.1	-30256.0	-1554.0
0.375	79482.8	10878.9	-23914.0	-3381.0
0.500	79638.4	16634.0	-20192.0	-2686.0
0.625	77887.0	19867.6	-16300.0	-3392.0
0.750	72556.4	19706.7		
0.875	64863.2	17433.1		
1.000	61646.0	19034.1		

TABLE XXVII. EXPERIMENTAL DATA SHALLOW NOTCH 70,000 lb LOAD

DISTANCE FROM NOTCH,in.	σ_θ ,psi	σ_r ,psi	NO LOAD RESIDUALS	
			σ_θ ,psi	σ_r ,psi
0.000	84748.9	8569.0	-50791.0	43.0
0.125	83492.9	9073.9	-46086.0	-736.0
0.250	84256.3	13716.2	-40774.0	-378.0
0.375	82846.8	13815.0	-34600.0	-1016.0
0.500	82524.6	16814.9	-30017.0	-1316.0
0.625	80446.4	17441.6	-24798.0	-1551.0
0.750	80455.5	23190.4		
0.875	76568.9	22938.7		
1.000	72264.1	23604.0		

TABLE XXVIII. EXPERIMENTAL DATA DEEP NOTCH LINEAR LOADING

15,000 lb Load

DISTANCE FROM NOTCH,in.	σ_{θ} ,psi	σ_r ,psi
0.000	45907.0	-45.0
0.125	35372.0	7693.0
0.250	30410.0	11671.0
0.375	26177.0	12395.0
0.500	20939.0	9879.0
0.625	18373.0	9060.0
0.750	16891.0	8984.0

TABLE XXIX. EXPERIMENTAL DATA DEEP NOTCH 30,000 lb LOAD

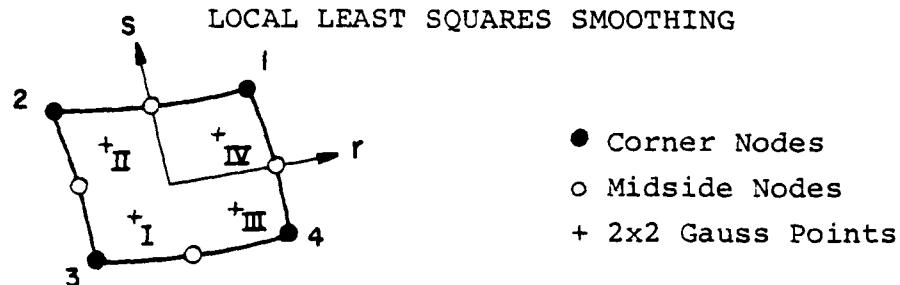
Elastic - Plastic

DISTANCE FROM NOTCH,in.	σ_{θ} ,psi	σ_r ,psi
0.000	76156.6	2587.4
0.125	63996.2	6057.7
0.250	50047.3	10456.3
0.375	43992.3	14485.3
0.500	39748.6	16433.6
0.625	35030.6	15899.3
0.750	33286.8	16728.0

APPENDIX A
PSAP1 JCL

```
----- STANDARD JCB CARD -----
// EXEC FRTXCLGP
//FORT.SYSPRINT DD DUMMY
//FORT.SYSIN DD UNIT=3330,VOL=SER=DISK02,
// DSN=S2939.PSAP(PSAP),DISP=SHR,LABEL=(,,IN)
// DD UNIT=3330,VOL=SER=DISK02,DSN=S2939.PSAP(PLOT),
// DISP=SHR,LABEL=(,,IN)
// DD UNIT=3330,VOL=SER=DISK02,DSN=S2939.PSAP(INIT),
// DISP=SHR,LABEL=(,,IN)
// DD UNIT=3330,VOL=SER=DISK02,DSN=S2939.PSAP(ELER),
// DISP=SHR,LABEL=(,,IN)
// DD UNIT=3330,VOL=SER=DISK02,DSN=S2939.PSAP(SAPF),
// DISP=SHR,LABEL=(,,IN)
// DD UNIT=3330,VOL=SER=DISK02,DSN=S2939.PSAP(ADNA),
// DISP=SHR,LABEL=(,,IN)
// DD UNIT=3330,VOL=SER=DISK02,DSN=S2939.PSAP(AUXL),
// DISP=SHR,LABEL=(,,IN)
// DD UNIT=3330,VOL=SER=DISK02,DSN=S2939.PSAP(ADPT),
// DISP=SHR,LABEL=(,,IN)
// DD *
C *** MAIN PROGRAM ***
      DIMENSION ZZZ(3203),DISPD(5,3,800)
      CALL PSAP1(ZZZ,3203,DISPO,800)
      STOP
      END
***** DELIMITER CARD (/*) *****
//GO.FT10F001 DD UNIT=SYSDA,DISP=(,PASS),
// SPACE=(CYL,(2,2)),DSN=&TEMP1
//GO.SYSIN DD *
***** INSERT PSAP1 DATA HERE *****
***** DELIMITER CARD (/*) *****
```

APPENDIX B



Two-Dimensional Isoparametric Element from ADINA [Ref. 4]

The local smoothing expression from Hinton and Campbell [Ref. 19] in ADINA coordinates becomes

$$\begin{Bmatrix} \check{\sigma}_1 \\ \check{\sigma}_2 \\ \check{\sigma}_3 \\ \check{\sigma}_4 \end{Bmatrix} = \begin{bmatrix} C & B & B & A \\ B & A & C & B \\ A & B & B & C \\ B & C & A & B \end{bmatrix} \times \begin{Bmatrix} \sigma_I \\ \sigma_{II} \\ \sigma_{III} \\ \sigma_{IV} \end{Bmatrix}$$

where $A = 1 + \frac{\sqrt{3}}{2}$, $B = -\frac{1}{2}$ and $C = 1 - \frac{\sqrt{3}}{2}$.

With $\check{\sigma}_1$, $\check{\sigma}_2$, $\check{\sigma}_3$ and $\check{\sigma}_4$ representing the smoothed corner node stresses and σ_I , σ_{II} , σ_{III} , and σ_{IV} as the unsmoothed stresses at the Gauss integration points, this expression can be written in an equivalent form.

$$\begin{Bmatrix} \check{\sigma}_3 \\ \check{\sigma}_4 \\ \check{\sigma}_1 \\ \check{\sigma}_2 \end{Bmatrix} = \begin{bmatrix} A & B & C & B \\ B & A & B & C \\ C & B & A & B \\ B & C & B & A \end{bmatrix} \times \begin{Bmatrix} \sigma_I \\ \sigma_{III} \\ \sigma_{IV} \\ \sigma_{II} \end{Bmatrix}$$

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AN ELASTIC-PLASTIC FINITE ELEMENT ANALYSIS OF NOTCHED ALUMINUM —ETC(U)
MAR 81 M J KAISER

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2 of 2

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The midside node stress values may be obtained by averaging the values at the associated corner nodes, since the distribution of the smoothed stresses is linear along the sides of the element. Smoothed stress values obtained by this least squares method should subsequently be averaged to obtain unique values at nodal points shared by adjacent elements.

APPENDIX C

ADINA JCL

```

----- STANDARD JCB CARD -----
// EXEC FORTXCLG,REGION=2000K
//FORT.SYSPRINT DD DJ44Y
//FORT.SYSIN DD *
      IMPLICIT REAL*8(A-H,O-Z)
      REAL A
      COMMON A(120010)
      MAX=120000
      CALL EXEC(MAX)
      STOP
      END
***** DELIMITER CARD /* *****
//LKED.JSDD DD DISP=SHR,DSN=MSS.S2939.ADINA
//LKED.SYSIN DD *
      INCLUDE USDD(LOADM)
      ENTRY MAIN
***** DELIMITER CARD /* *****
//GO.FT07F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
// DCB=(RECFM=VBS,BLKSIZE=4000),SPACE=(CYL,(5,1))
//GO.FT01F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
// DCB=(RECFM=VBS,BLKSIZE=4000),SPACE=(CYL,(5,1))
//GO.FT02F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
// DCB=(RECFM=VBS,BLKSIZE=4000),SPACE=(CYL,(5,1))
//GO.FT03F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
// DCB=(RECFM=VBS,BLKSIZE=1000),SPACE=(CYL,(5,1))
//GO.FT04F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
// DCB=(RECFM=VBS,BLKSIZE=1000),SPACE=(CYL,(5,1))
//GO.FT08F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
// DCB=(RECFM=VBS,BLKSIZE=4000),SPACE=(CYL,(5,1))
//GO.FT09F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
// DCB=(RECFM=VBS,BLKSIZE=4000),SPACE=(CYL,(5,1))
//GO.FT10F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
// DCB=(RECFM=VBS,BLKSIZE=4000),SPACE=(CYL,(5,1))
//GO.FT11F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
// DCB=(RECFM=VBS,BLKSIZE=1000),SPACE=(CYL,(5,1))
//GO.FT12F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
// DCB=(RECFM=VBS,BLKSIZE=1000),SPACE=(CYL,(5,1))
//GO.FT13F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
// DCB=(RECFM=VBS,BLKSIZE=1000),SPACE=(CYL,(5,1))
//GO.FT56F001 DD UNIT=3330,VOL=SER=DISK01,
// DSN=F0099. TEMP,DISP=SHR,LABEL=(,,IV)
//GO.FT57F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
// DCB=(RECFM=VBS,BLKSIZE=1000),SPACE=(CYL,(5,1))
//GO.SYSIN DD *
***** INSERT ADINA DATA HERE *****
***** DELIMITER CARD /* *****
--- BLANK CARD ---
--- BLANK CARD --- (TWO BLANK CARDS STOP EXEC)
***** DELIMITER CARD /* *****

```

PSAPI MAR 1981 AS MODIFIED BY LCDR H.J. KAISER

SUBROUTINE PSAPI DOCUMENTATION

DESCRIPTION OF INPUT DATA CARDS

TITLE CARD - 80 ALPHANUMERIC CHARACTERS OF GRAPH TITLE INFORMATION
TO BE PRINTED ABOVE THE GRAPH. THE FIRST 40 CHARACTERS WILL FORM THE FIRST TITLE LINE. THE LAST 40 CHARACTERS WILL FORM THE SECOND TITLE LINE.

NAMELIST OPTION - CONTAINS VALUES TO VERIFY STORAGE IN BLANK
COMMON AND CONTROL VALUES NEEDED BY THE PROGRAM.

THE FOLLOWING VALUES ARE INCLUDED---

NINDEST = ESTIMATE NUMBER OF GRID POINTS TO BE USED. VALUE MUST
BE GREATER THAN OR EQUAL TO THE ACTUAL NUMBER OF GRID

POINTS.
** DEFAULT = 200 **
NUDISP = 0 FOR NO DISPLACEMENT DATA IN X-DIRECTION.
** = 1 FOR DATA INCLUDING DISPLACEMENTS IN X-DIRECTION.
NWDISP = 0 FOR NO DISPLACEMENT DATA IN Y-DIRECTION.
** = 1 FOR DATA INCLUDING DISPLACEMENTS IN Y-DIRECTION.
** DEFAULT = 0 **
NWDISP = 0 FOR NO DISPLACEMENT DATA IN Z-DIRECTION.
** = 1 FOR DATA INCLUDING DISPLACEMENTS IN Z-DIRECTION.
** DEFAULT = 0 **

KGEOM SPECIFIES SUBROUTINE AND CORRESPONDING METHOD OF INPUT
FOR MODEL GEOMETRY.
KGEOM = 1 FOR USER SUPPLIED SUBROUTINE - GEOM1
= 2 FOR USER SUPPLIED TO READ ADINA GEOMETRY DATA - MAR 77
= 3 FOR SAP IV DATA DECK INPUT SUBROUTINE - GEOM2
= 9 FOR SAP IV DATA DECK INPUT SUBROUTINE - GEOM9 MAR 77
** GEOM9 READS SAP IV GEOMETRY DATA - MODIFIED
** DEFAULT = 9 **
KDATA SPECIFIES SUBROUTINE AND CORRESPONDING METHOD OF INPUT
FOR DISPLACEMENT DATA.
KDATA = 1 FOR SUBROUTINE DATA1 TO READ IN DISPLACEMENT DATA
-- SUPPLIED BY THE USER.

APR1981
DDCU00020
DDCU00030
DDCU00040
DDCU00050
DDCU00060
DDCU00070
DDCU00080
DDCU00090
DDCU0100
DDCU0110
DDCU0120
DDCU0130
DDCU0140
DDCU0150
DDCU0160
DDCU0170
DDCU0180
DDCU0190
DDCU0200
DDCU0210
DDCU0220
DDCU0230
DDCU0240
DDCU0250
DDCU0260
DDCU0270
DDCU0280
DDCU0290
DDCU0300
DDCU0310
DDCU0320
DDCU0330
DDCU0340
DDCU0350
DDCU0360
DDCU0370
DDCU0380
DDCU0390
DDCU0400
DDCU0410
DDCU0420
DDCU0430
DDCU0440
DDCU0450
DDCU0460
DDCU0470
DDCU0480

```

= 5 FOR SUBROUTINE DATA5 TO READ IN DISPLACEMENT DATA
-- SUPPLIED BY THE USER.
= 9 FOR SUBROUTINE DATA9 TO READ SAP IV DATA.
** DEFAULT = 9 **

INVALUS - NOT USED AT NPS ----- ALLOW DEFAULT

*** DEFAULT = 0 *** NOT USED AT NPS ----- ALLOW TO DEFAULT
*** DEFAULT = 1 *** SPECIFIES THE TYPE OF OUTPUT DEVICE TO BE USED.

KPLOT = 1 FOR CALCOMP.
= 2 FOR Langley Research Center USE ONLY
= 3 FOR LRC USE ONLY
= 4 FOR LRC USE ONLY
*** DEFAULT = 1 *** SPACE BETWEEN PLOTS IN Y DIRECTION (INCHES) WHEN
MUTIPLE PLOTS ARE PRODUCED. YSPACE/2.0 IS SPACE
BETWEEN TITLE BLOCK AND PLOT.

*** DEFAULT = 2.0 ***
PSIZE = PAPER SIZE IN X DIRECTION USED IN SCALING OF
PLOTS TO INSURE THIS DIMENSION IS NOT EXCEEDED.

*** DEFAULT = 9.0 ***
IDCASE = 0 FOR NO TITLE CARD PRECEDING
DECKS OF DISPLACEMENT VALUES.
= 1 DECKS OF DISPLACEMENT VALUES.
*** DEFAULT = 0 ***

```

MODEL GEOMETRY IS NOW INPUT IN ONE OF THE FOLLOWING FORMS, DEPENDING ON THE VALUE OF KGEOM SPECIFIED IN NAMELIST OPTION.

```

USE IF KGEOM = 1 CALL SUBROUTINE GEOM1 WHICH READS ADINA GEOMETRY DATA
USE IF KGEOM = 2 CALL SUBROUTINE GEOM2 WHICH IS PREPARED BY THE USER TO READ GEOMETRY DATA.

```

USE IF KGEM = 9
CALL SUBROUTINE GEOM9 WHICH READS SAP IV GEOMETRY DATA.

CASE IDENTIFICATION CARD.

THIS CARD IS OMITTED IF IDCASE=0 IS SPECIFIED IN EOPTION
IF PRESENT, THIS CARD CONTAINS ANY DESIRED ALPHANUMERIC
INFORMATION IN COLS 1-80 WILL NOT APPEAR ON PLOT BUT WILL
APPEAR ON PRINTOUT ABOVE DISPLACEMENT DATA

DATA TO BE PLOTTED IS NOW INPUT IN ONE OF THE FOLLOWING FORMS:
DEPENDING ON THE VALUE OF KDATA SPECIFIED IN NAMELIST OPTION.

USE IF KDATA = 1
CALL SUBROUTINE DATA1 WHICH IS PREPARED BY THE USER

USE IF KDATA = 5
CALL SUBROUTINE DATA5 WHICH IS PREPARED BY THE USER

CALL SUBROUTINE DATA9 WHICH READS SAP IV DISPLACEMENT DATA.
A DISPLACEMENT DATA DECK CAN BE PREPARED FOR ADINA IN A
FORMAT COMPATABLE WITH DATA9.

NAMELIST PICT - CONTAINS VALUES NEEDED TO GENERATE PLOTS.
THE FOLLOWING VALUES ARE INCLUDED---

KHORZ = INTEGER DESIGNATING HORIZONTAL AXIS OF VIEWING PLANE,
WHERE 1=X 2=Y, 3=Z.
** DEFAULT = 1 **
KVERT = INTEGER DESIGNATING VERTICAL AXIS OF VIEWING PLANE,
WHERE 1=X 2=Y, 3=Z.
** DEFAULT = 2 **
PHI = ANGULAR ROTATION OF MODEL ABOJT ITS X-AXIS, IN DEGREES
(MUST BE TAKEN THIRD).
** DEFAULT = 0 **
THETA = ANGULAR ROTATION OF MODEL ABOUT ITS Y-AXIS, IN DEGREES
(MUST BE TAKEN SECOND).
** DEFAULT = 0 **
PSI = ANGULAR ROTATION OF MODEL ABOUT ITS Z-AXIS, IN DEGREES

DDCU0970
DDCU0980
DDCU1000
DDCU1010
DDCU1020
DDCU1030
DDCU1040
DDCU1050
DDCU1060
DDCU1070
DDCU1080
DDCU1090
DDCU1100
DDCU1110
DDCU1120
DDCU1130
DDCU1140
DDCU1150
DDCU1160
DDCU1170
DDCU1180
DDCU1190
DDCU1200
DDCU1210
DDCU1220
DDCU1230
DDCU1240
DDCU1250
DDCU1260
DDCU1270
DDCU1280
DDCU1290
DDCU1300
DDCU1310
DDCU1320
DDCU1330
DDCU1340
DDCU1350
DDCU1360
DDCU1370
DDCU1380
DDCU1390
DDCU1400
DDCU1410
DDCU1420
DDCU1430
DDCU1440

*# DEFAULT = 0.0**
 NEWFR = 1 FOR FRAME CHANGE BEFORE PLOT IS MADE.
 (A BY YSPACE AND X-ORIGIN AT 0.0)
 NEWFR.NE.1 FOR NO FRAME CHANGE BEFORE PLOTTING
 *# ISCALE = 1 FOR INTERNAL ORIGIN LOCATION AND SCALING.
 = 2 FOR USER SPECIFIED ORIGIN AND SCALING.
 = 0 FOR NO SCALE CHANGE (I.E. USE SAME SCALE AS PREVIOUS PLOT).
 THIS IS USEFUL IN AN ASSEMBLY GRAPH WHERE IT IS NECESSARY TO EXAMINE A MESH IN SECTIONS WITHOUT LOSING THE FIRST PLOT.
 *# PLOTSZ = MAXIMUM DIMENSION DESIRED ON COMPLETED PLOT.
 (USES FOR SCALING IF ISCALE = 1)
 PLOTSZ SCALES THE PLOT PRIOR TO ROTATION. IF ROTATION CAUSES THE PLOT TO EXCEED PAPER WIDTH (PSIZE), IT IS RESCALED AND THE PLOT SIZE IS REDUCED ACCORDINGLY.
 *# DEFAULT = 10.0 **
 XLOCN = X-LOCATION OF PLOT ORIGIN (USED IF ISCALE = 2).
 YORGN = Y-LOCATION OF PLOT ORIGIN (USED IF ISCALE = 2).
 *# PSCALE = MODEL SIZE/DESIRED PLOT SIZE (USED IF ISCALE = 2).
 *# NOTAT = 0 FOR NO NUMBERING ON PLOTS.
 = 1 FOR NUMBERING OF GRID POINTS.
 = 2 FOR NUMBERING OF ELEMENTS.
 *# XLHT = HEIGHT OF INTEGERS SPECIFIED BY NOTAT, IN INCHES.
 KDISP = 0.15**
 KDISP = 0 FOR UNDEFORMED PLOT.
 = 1 FOR DEFORMED PLOT.
 = 2 FOR EXPLODED PLOT.
 = 3 FOR DISPLACEMENTS REPRESENTED BY VECTORS.
 *# IDMAG = 0**
 = 1 FOR DIRECT SCALING OF DATA BY DMAGS.
 = 2 FOR SCALING OF DATA TO A MAX. VALUE OF DMAGS.
 *# DMAGS = MAGNIFICATION OF DISPLACEMENTS (IF KDISP=1).
 = REDUCTION FACTOR OF ELEMENTS (IF KDISP=2).
 *# KSYMXY = 1 FOR SYMMETRY ABOUT X-Y PLANE.
 KSYMXZ = 1 FOR SYMMETRY ABOUT X-Z PLANE.
 *# DEFAULT = 0**

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KSYMYZ = 1 FOR SYMMETRY ABOUT Y-Z PLANE.
** DEFAULT = 0 **
XXMAX YYMAX ZZMAX XXMIN YYMIN ZZMIN LOCATE CUTTING PLANES
PARALLEL TO PRINCIPAL (X-Y,X-Z,Y-Z) PLANES
TO LIMIT PLOT.
** DEFAULT XXMAX=YYMAX=ZZMAX=1.0E+20 **
** DEFAULT XXMIN=YYMIN=ZZMIN=-1.0E+20 **
NDMAX = MAXIMUM GRID PT. TO BE INCLUDED IN PLOT.
** DEFAULT = 9999999999999999 **
NDMIN = MINIMUM GRID PT. TO BE INCLUDED IN PLOT.
** DEFAULT = 0 **
NELMAX = MAXIMUM ELEMENT NUMBER TO BE INCLUDED IN PLOT.
** DEFAULT = 9999999999999999 **
NELMIN = MINIMUM ELEMENT NUMBER TO BE INCLUDED IN PLOT.
** DEFAULT = 0 **
KODE SPECIFIES CONTROL OPTION AFTER PLOT IS COMPLETE.
KODE = 0, LAST PLOT, EXIT FROM PROGRAM.
= 1, READ ANOTHER NAMELIST PICT.
= 2, READ A NEW SET OF DISPLACEMENT DATA, INCLUDING A
CASE IDENTIFICATION CARD IF PRESENT.
= 3, READ A COMPLETE NEW SET OF INPUT DATA,
** INCLUDING A TITLE CARD.
** DEFAULT = 0 **

*****
```

THE ABOVE COMPRISES A COMPLETE BASIC SET OF INPUT DATA IF
 KODE = 0 IN EPIC. FOR KODE = 1, 2, OR 3, ADDITIONAL SECTIONS OF
 THE BASIC DECK MUST BE REPEATED. THE DECK MUST END WITH
 NAMELIST EPIC HAVING KODE = 0.

 SUBROUTINE PSAPI IS A MODIFICATION TO NAVAL POSTGRADUATE
 SCHOOL THESIS BY LT. D. L. LOUGH, DECEMBER 1976. MODIFICATION
 INCLUDED SAP IV 8-21 NODE BRICK ELEMENTS, BOUNDARY ELEMENTS AND
 ADIVA TRUSS, PLANE, BRICK, BEAM ELEMENTS, AND OTHER MINOR
 IMPROVEMENTS.

MODIFIED BY ADRIAN E. KIBLER JR.
 LT USN
 NAVAL POSTGRADUATE SCHOOL
 MONTEREY, CA
 JAN - JUN 1977

DOCU2560
DOCU2570
#4P81981

ISOLATED GRAPHICS PLOTTING SUBROUTINES

THE SUBROUTINES USED IN THE ACTUAL CREATION OF PLOTS BY THE VERSATEC MODEL 8222A ARE EXPLAINED IN NPS TECHNICAL NOTE NUMBER 0141-34, "USING VERSATEC PLOTTER AT NPS".

PSAB1 SUBORDINATES WITH VERSATILE CALL STATEMENTS

NOTATE

CALCUP	CALPLT	CALNUM
CALWH	CALINE	PSTOP

* * * * *

EXAMPLE OF MAIN PROGRAM USED TO ALLOCATE CORE STORAGE

```

DIMENSION ZZZ(NZ),DISPD(5,3,NON)
CALL PSAPI(ZZZ,NZ,DISPD,NON)
STOP
END

```

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NZ---MUST BE GREATER THAN * NON +
17*NON+3 IF DISPLACEMENT DATA IS INPUT


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C *** TO ZERO NODE AND ELEMENT SUMMATION COUNTERS
C
C ILOOP = 0
C NNODE = 0
C YPMAX=0.0
C
C *** TO DEFINE THE ORIGIN AND OPEN PLOTING DATA SETS
C
C CALL CALCMA
C
C 500 CONTINUE
C REWIND 10
C WRITE(6,18)
C   18 FORMAT(1H1)
C
C *** TO READ TITLE CARD FOR RIN
C
C   READ(5,9004,END=999) (ABCD1(I),I=1,10), (ABCD2(I),I=1,10)
C   9004 FORMAT(20A4)
C   WRITE(6,9006) (ABCD1(I),I=1,10), (ABCD2(I),I=1,10)
C   9006 FORMAT(//,20X,20A4//)
C   CALL INITIAL
C
C *** TO PLOT THE TITLE CARD AT THE BEGINING OF THE PLOT
C
C   CALL CALPLT(0.0,0.62,3)
C   CALL CALPLT(0.0,0.0,2)
C   CALL CALPLT(9.0,0.0,2)
C   CALL NOTATE(0.8,0.41,0.21,ABCD1,0.0,40)
C   CALL NOTATE(0.8,0.10,0.21,ABCD2,0.0,40)
C   CALL CALPLT(0.3,1.62+YSPACE/2.0,-3)
C
C *** TO SET POINTERS FOR BLANK COMMON STORAGE ZZZ SUBROUTINES
C
C NUMPT = 1
C XPT = NUMPT+NNDEST
C YPT = XPT+NNDEST
C ZPT = YPT+NNDEST
C UPT = ZPT+NNDEST
C
C IF(NUDISP.EQ.0) VPT = UPT+1
C IF(NUDISP.NE.0) VPT = UPT+NNDEST
C IF(NVDISP.EQ.0) WPT = VPT+1
C IF(NVDISP.NE.0) WPT = VPT+NNDEST
C IF(NWDISP.EQ.0) NEND = WPT+1-1
C IF(NWDISP.NE.0) NEND = WPT+NNDEST-1
C
C WRITE(6,15) NEND
C
C 15 FORMAT(//,20X, *BLANK COMMON STORAGE ZZZ REQUIRES AT LEAST *,16,

```

```

1 LOCATIONS FJR THIS CASE ///
1 IF(KGEDM.EQ.1) CALL GEOM1
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0800
1 ZZZ(GEOM.EQ.2) CALL GEOM2
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0820
1 ZZZ(GEOM.EQ.9) CALL GEOM9
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0840
1 CALL PNTDUT ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0850
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0860
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0870
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0880
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0890
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0900
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0910
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0920
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0930
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0940
1 IF(KDATA.EQ.1) CALL DATA1
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0950
1 ZZZ(KDATA.EQ.5) CALL DATA5
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0960
1 ZZZ(KDATA.EQ.9) CALL DATA9
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0970
1 ZZZ(KDATA.EQ.13) CALL DATA13
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0980
1 ZZZ(KDATA.EQ.17) CALL DATA17
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP0990
1 ZZZ(KDATA.EQ.21) CALL DATA21
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP1000
20 IF(NUDISP.EQ.0 .AND. NWDISP.EQ.0) GO TO 700
CALL PNTDUT ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP1030
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP1040
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP1050
1 ZZZ(NUMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP1060
700 CONTINUE
IF(KPLOT.EQ.4 .AND. ILOOP.NE.0) GO TO 6000
WRITE(6,1000)
1000 FORMAT(//)
READ(5,PICT)
WRITE(6,PICT)
6000 CONTINUE
CALL DSCALE
1 ZZZ(NJMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP1130
1 CALL BOUND
1 ZZZ(NJMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP1140
1 IF(LSCALE.NE.0) CALL ROTAT
1 CALL PLOTX
1 ZZZ(NJMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP1160
1 IF(LLOOP=1)LOOP+1
1 ZZZ(NJMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP1180
1 ZZZ(NJMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP1190
1 ZZZ(NJMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP1200
1 ZZZ(NJMPY),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT) PSAP1210
C *** TO PLOT TITLE ON TOP OF GRAPH IF KODE = 3
C *** TO PLOT TITLE IN TOP AND CLOSE PLOTING DATA SETS IF KODE = 0
C CALL CALPLT(0.0, YMAX+YSPACE/2.0,-3)
C CALL CALPLT(0.0, 1.0, 3)
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CALL CALPLT(0.0, 1.62, 2)
CALL CALPLT(9.0, 1.62, 2)
CALL NOTATE(0.8, 1.31, 21, ABCD1, 0, 40)
CALL NOTATE(0.8, 1.0, 21, ABCD2, 0, 40)
CALL CALPLT(0.0, 1.62, YSPACE, -3, 0, 40)

ILOOP=0
IF(KODE .EQ. 31) GO TO 500
WRITE(6,9008)
FORMAT(1/'%5X', TERMINATION NORMAL DUE TO KODE = 0')
CALL PSTOP
RETURN
999 CALL ERORR(2)
END
SUBROUTINE PLTIX( NUMPT, XPT, YPT, ZPT, UPT, VPT, WPT )
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C *** FOR GENERATING PLOTS.
C *** CALLED BY PSAPI
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
COMMON/CONTROL/ KGEOM, KDATA, KPLOT, KSYMXY, KSYMZX, KSYMZY, NOTAT, XLHT,
1KHORZ, KVERT, PHI, THETA, PSI, NEWFR, ISCALE, PLOTSZ, KORGN, YORGN,
2PSCALE, KDISP, D4AG, KODE
COMMON/LIMITS/ XMAX, YMAX, ZMAX, XXMIN, YYMIN, ZZMIN, NDMAX,
INELMAX, NELMIN
COMMON/XYZLIN/ XYZMAX(3), XYZMIN(3)
COMMON/CORGN/ YPMAX, YPMIN, PSIZE
COMMON/GLOOP/ ILOOP
COMMON/ABLK/ A(3,3)
COMMON/KOUNT/ NNODE, NNODEST, NU DISP, NW DISP
COMMON/PDELS/ DELX, DELY
DIMENSION NUMPT(1), XPT(1), YPT(1), ZPT(1), UPT(1), VPT(1), WPT(1),
DIMENSION NODE(20), X(20), Y(20), Z(20), U(20), V(20), W(20),
IDISP(20), XROT(20), YROT(20), ZROT(20), UP(23), VP(23)
C *** TO MAKE ALL GRID POINT NUMBERS NEGATIVE
C DO 50 I=1,NNODE
      IF(NUMPT(I).GT.0) NUMPT(I)=-NUMPT(I)
50 CONTINUE
C *** TO MAKE FRAME CHANGE IF NEWFR = 1 AFTER NAMELIST OPTION
C *** NO FRAME CHANGE ON FIRST PLOT
C YMOVE=0.0

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```

      IF(ILOOP.EQ.0) GO TO 70
      IF(NEWFR.EQ.1) YMOVE=YPMAX+YSPACE
    70 CALL CALPL(0.0,YMOVE,-3)
      GO TO (710,710,710),K PLOT
    703 CONTINUE

C 710 CONTINUE
      IF(ISCALE.NE.0) DELX=0.0
      IF(ISCALE.NE.0) DELY=0.0
      IF(ISCALE.EQ.1) CALL XYSCAL
      CALL CALPL(XORGN,YORGN,-3)
      XSHIFT = 0.0
      XYSHIFT = 0.0
      ZSHIFT = 0.0
      YPMAX=-1.0E20

C *** LOOPS TO ACCOUNT FOR SYMMETRY
      ZSIGN = +1.0
      DO 500 II=1,2
      IF(II.EQ.2.AND.KSYMXY.NE.1) GO TO 500
      IF(II.EQ.0.AND.KSYMXY.EQ.1) ZSIGN = -1.0
      YSIGN = +1.0
      DO 510 JJ=1,2
      IF(JJ.EQ.2.AND.KSYMXX.NE.1) GO TO 510
      IF(JJ.EQ.0.AND.KSYMXX.EQ.1) YSIGN = -1.0
      XSIGN = +1.0
      DO 520 KK=1,2
      IF(KK.EQ.2.AND.KSYMYY.NE.1) GO TO 520
      IF(KK.EQ.0.AND.KSYMYY.EQ.1) XSIGN = -1.0

C *** TO DETERMINE PROJECTED COORDINATES OF ELEMENTS
      REWIND 10
    100 CONTINUE
      READ(10,END=1000) NEND,NUMEL,(NODE(j),j=1,NELMAX)
      DO 10 I=1,NEND
      ND = NODE(I)
      IF(NODE(I).EQ.0) GO TO 10

C *** TO MAKE GRID POINT NUMBERS CONNECTED BY ELEMENTS POSITIVE
      NUMPT(ND) = ABS(NUMPT(ND))
      IF(NUMPT(ND).LT.NDMIN.OR.NUMPT(ND).GT.NDMAX) GO TO 100
    10 CONTINUE
      I = KHORZ
      J = KVVERT
      DO 20 N=1,NEND

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```

IF(NODE(N).EQ.0) GO TO 20
ND = NODE(N)
IF(XPT(ND).LT.XXMAX) GO TO 100
IF(YPT(ND).LT.YYMAX) GO TO 100
IF(ZPT(ND).LT.ZZMAX) GO TO 100
IF(ZPT(ND).LT.ZZMIN) GO TO 100
XDISP(N) = 0.0
YDISP(N) = 0.0
ZDISP(N) = 0.0
IF(KDISP.EQ.1.AND.NUDISP.NE.0) XDISP(N) = UPT(ND)
IF(KDISP.EQ.1.AND.NUDISP.NE.0) YDISP(N) = VPT(ND)
IF(KDISP.EQ.1.AND.NUDISP.NE.0) ZDISP(N) = WPT(ND)
X(N) = XSIGN*(XPT(ND)*XDISP(N)*DMAG+XSHIFT)/PSCALE
Y(N) = YSIGN*(YPT(ND)*YDISP(N)*DMAG+YSHIFT)/PSCALE
Z(N) = ZSIGN*(ZPT(ND)*ZDISP(N)*DMAG+ZSHIFT)/PSCALE
CONTINUE
20 IF(KDISP.EQ.2) CALL XPLOD(NEND,X,Y,Z,NODE)
XCENT = 0.0
YCEN = 0.0
FN0=0.0
DO 25 N=1,NEND
IF(NODE(N).EQ.0) GO TO 25
XR0T(N) = A(I,1)*X(N)+A(I,2)*Y(N)+A(I,3)*Z(N)
YR0T(N) = A(J,1)*X(N)+A(J,2)*Y(N)+A(J,3)*Z(N)
IF(N.GT.8) GO TO 24
FND=FND+1.0
XCENT = XCEN+XR0T(N)
YCEN = YCEN+YR0T(N)
CONTINUE
24 XR0T(N) = XR0T(N)+DELY
YR0T(N) = YR0T(N)+DELY
IF(YR0T(N).GT.YMAX) YMAX=YR0T(N)
25 CONTINUE
IF(NOTAT.NE.2) GO TO 29
XCENT = XCEN/FND-(6.0/7.0)*XLHT
YCEN = YCEN/FND-XLHT/2.0
XCENT = XCEN+DELY
YCEN = YCEN+DELY
AL = NUME
CONTINUE
29 IF(NDITAT.EQ.2) CALL CALNUM(XCENT,YCEN,XLHT,AL,0.0,-1)
C *** TO PLOT ELEMENTS
C
IF(NEND.EQ.2) 50 TO 280
IF(NEND.EQ.4) 30 TO 300
PL0T0830
PL0T0840
PL0T0860
PL0T0870
PL0T0880
PL0T0890
PL0T0900
PL0T0910
PL0T0920
PL0T0930
PL0T0940
PL0T0950
PL0T0960
PL0T0970
PL0T0980
PL0T0990
PL0T1000
PL0T1010
PL0T1020
PL0T1030
PL0T1040
PL0T1050
PL0T1060
PL0T1070
PL0T1080
PL0T1090
PL0T1100
PL0T1110
PL0T1120
PL0T1130
PL0T1140
PL0T1150
PL0T1160
PL0T1170
PL0T1180
PL0T1190
PL0T1200
PL0T1210
PL0T1220
PL0T1230
PL0T1240
PL0T1250
PL0T1260
PL0T1270
PL0T1280
PL0T1290
PL0T1300

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IF(NEND.EQ.8) GO TO 320
IF(NEND.EQ.12) GO TO 340
IF(NEND.EQ.20) GO TO 340
CALL ERROR(4)

C ***TO PLOT 2 NODE ELEMENT
C 280 CONTINUE
CALL CALPLT(XRDT(1),YROT(1),3)
CALL CALPLT(XRDT(2),YROT(2),2)
GO TO 430

C *** TO PLOT 3 AND 4 NODE PLANE ELEMENT
C 300 CONTINUE
CALL CALPLT(XRDT(1),YROT(1),3)
DO 305 NP=2,NEND
CALL CALPLT(XRDT(NP),YROT(NP),2)
CONTINUE
CALL CALPLT(XRDT(1),YROT(1),2)
GO TO 430

C *** TO PLOT 8 NODE 3-D BRICK
C 320 CONTINUE
LP=1
DO 330 NP=2,6,4
NP2=NP+2
CALL CALPLT(XRDT(LP),YROT(LP),3)
DO 325 NP=NP NP2
CALL CALPLT(XRDT(NP),YROT(NP),2)
CONTINUE
CALL CALPLT(XRDT(LP),YROT(LP),2)
LP=LP+4
CONTINUE
DO 335 NP=1,4
NP4=NP+4
CALL CALPLT(XRDT(NP),YROT(NP),3)
CALL CALPLT(XRDT(NP4),YROT(NP4),2)
CONTINUE
335 GO TO 430

C *** TO PLOT VARIABLE 4-8 NODE PLANE AND 8-20 NODE BRICK ELEMENTS
C 340 CONTINUE
LP=1
KP=8
DO 365 NP=2,6,4

```

```

NP2=NP+2
CALL CALPLT(XROT(LP), YROT(LP), 3)
DO 345 NP=NP, NP2
  KP=KP+1
  N=2
  CALL CALWH(XP(1), YP(1))
  XP(2)=XROT(MP)
  YP(2)=YROT(MP)
  XP(3)=XROT(KP)
  YP(3)=YROT(KP)
  IF(NODE(KP).NE.0) CALL CURVE(XP, YP, N)
  CALL CALINE(XP, YP, N)
  CONTINUE
345  KP=KP+1
  N=2
  CALL CALWH(XP(1), YP(1))
  XP(2)=XROT(LP)
  YP(2)=YROT(LP)
  XP(3)=XROT(KP)
  YP(3)=YROT(KP)
  IF(NODE(KP).NE.0) CALL CURVE(XP, YP, N)
  CALL CALINE(XP, YP, N)
  LP=LP+4
  IF(NEND.EQ.12) GO TO 430
  CONTINUE
355  DO 390 NP=1,4
    NP4=NP+4
    KP=NP+16
    N=2
    XP(1)=XROT(NP)
    YP(1)=YROT(NP)
    XP(2)=XROT(NP4)
    YP(2)=YROT(NP4)
    XP(3)=XROT(KP)
    YP(3)=YROT(KP)
    IF(NODE(KP).NE.0) CALL CURVE(XP, YP, N)
    CALL CALINE(XP, YP, N)
    CONTINUE
365  430  CONTINUE
      GO TO 100
    1000 CONTINUE
      IF(KDISP.NE.3) GO TO 650
    600 CONTINUE
C *** TO PLOT VECTORS AT GRID POINTS
C
      DO 601 ND=1, NNODE
        IF(NUMPT(ND).LE.0) GO TO 601

```

```

IF(NUMLT(ND).LT.NDMIN.ND).OR.NUMPT(ND).GT.NDMAX) GO TO 601
IF(XPT(ND).LT.XYZMIN(1)) GO TO 601
IF(XPT(ND).LT.XYZMAX(1)) GO TO 601
IF(YPT(ND).LT.XYZMIN(2)) GO TO 601
IF(YPT(ND).LT.XYZMAX(2)) GO TO 601
IF(ZPT(ND).LT.XYZMIN(3)) GO TO 601
IF(ZPT(ND).LT.XYZMAX(3)) GO TO 601
X(1)=XSIGN*(XPT(ND)+XSHIFT)/PSCALE
Y(1)=YSIGN*(YPT(ND)+YSHIFT)/PSCALE
Z(1)=ZSIGN*(ZPT(ND)+ZSHIFT)/PSCALE
XDISP(1)=0.0
YDISP(1)=0.0
ZDISP(1)=0.0
IF(NUDLSP.NE.0) XDISP(1)=UPT(ND)
IF(NVDLSP.NE.0) YDISP(1)=VPT(ND)
IF(NWDLSP.NE.0) ZDISP(1)=WPT(ND)
X(2)=XSIGN*(XPT(ND)+XDISP(1)*DMAG+XSHIFT)/PSCALE
Y(2)=YSIGN*(YPT(ND)+YDISP(1)*DMAG+YSHIFT)/PSCALE
Z(2)=ZSIGN*(ZPT(ND)+ZDISP(1)*DMAG+ZSHIFT)/PSCALE
I=KDRZ
J=KVER
DO 605 N=1,2
XROT(N)=A(I,1)*X(N)+A(I,2)*Y(N)+A(I,3)*Z(N)
YROT(N)=A(J,1)*X(N)+A(J,2)*Y(N)+A(J,3)*Z(N)
XROT(N)=XROT(N)+DELY
YROT(N)=YROT(N)+DELY
605 CONTINUE
XARW=0.06
YARW=XARW/3.0
CALL GARRW(XROT(1),YROT(1),XROT(2),YROT(2),XARW,YARW)
C *** TO PLOT NODE POINT NUMBERS
C IF(NODET.EQ.1) CALL NODET(NUMLT,XPT,YPT,ZPT,UPT,VPT,WPT)
C CALL CALPLT(-XORGN,-YORGN,-3)
C *** TO MAKE ALL GRID POINT NUMBERS POSITIVE AGAIN
C DO 1100 I=1,NODE
NUMPT(I)=IAS(NUMLT(I))
1100 CONTINUE
RETURN
END

```

```

      SUBROUTINE CURVE(XP,YP,N)
C *   *   *   *   *   *   *   *   *   *   *   *   *   *   *   *   *   *
C *** THIS SUBROUTINE INTERPOLATES ALONG THE EDGES OF ISOPARAMETRIC
C *** ELEMENTS USING SHAPE FUNCTIONS CALLED BY PLOTX
C *   *   *   *   *   *   *   *   *   *   *   *   *   *   *   *   *   *
      DIMENSION XP(11),YP(11)
      DIMENSION X(3),Y(3)
      DO 100 I=1,3
         X(I)=XP(I)
         Y(I)=YP(I)
      100   CONTINUE
      R=-1.0
      DO 200 I=1,21
         YP(I)=Y(1)+R*(Y(3)-Y(1))/2.0-Y(3)/2.0-X(1)+R*(X(3)-X(1))/2.0-X(3)/2.0
         R=R+0.1
      200   CONTINUE
      N=21
      RETURN
      END
      SUBROUTINE DSCALE(NUMPT,XPT,YPT,ZPT,UPT,VPT,WPT)
C *   *   *   *   *   *   *   *   *   *   *   *   *   *   *   *   *   *
C *** THIS SUBROUTINE DETERMINES THE SCALE FACTOR FOR DISPLACEMENTS
C *** CALLED BY PSAP1
C *   *   *   *   *   *   *   *   *   *   *   *   *   *   *   *   *   *
      COMMON/CONTROL/KGEOM,KDATA,KPLOT,KSYMXX,KSYMZZ,KNOTAT,XLHT,
     1KHORZ,KVERT,PHI,THETA,KPSI,NEWFR,ISCALE,PLOTSZ,XORGN,YORGN,
     2PSCALE,KDISP,DAG,KODE
      COMMON/KOUNT/NNODE,NNDEST,NUDISP,NWDISP,NYDISP
      COMMON/NUMPT/NUMPT(1),VPT(1),ZPT(1),UPT(1),WPT(1)
      IF(KDISP.EQ.0.D0.KDISP.EQ.2.D0) GO TO 10
      GO TO (10,20), IDMAG
      10 CONTINUE
      DMAG = DMAGS
      GO TO 30
      20 CONTINUE
      DMAG = 0.0
      DO 100 I=1,NNODE

```



```

C *** TO DETERMINE SCALE FACTOR FOR MODEL GEOMETRY.
C *** CALLED BY PLOTX
C *
C * * * * * COMMON/CONTRL/ KGEOM,KDATA,KPILOT,* * * * *
C * * * * * COMMON/KVERT,PHI,THETA,PSI,NEWFR,ISCALE,PLOTSZ,XLHT.
C * * * * * 1KHORZ,KVERT,PHI,THETA,PSI,NEWFR,ISCALE,PLOTSZ,XLHT.
C * * * * * 2PSCALE,KDISP,DMAX,KODE,SYMXY,SYMMZ,XORGN,YORGN,
C * * * * * COMMON/XYZLIM,XYZMAX(3),XYZMIN(3)
C * * * * * COMMON/YMAX,YSPACE,PSIZE
C * * * * * COMMON/CORGN,A(3,3)
C * * * * * COMMON/PDELS,DELX,DELY
C * * * * * 1 = KHORZ
C * * * * * 2 = KVERT
C * * * * * DMAX = 0.0
DO 5 N=1,3
YDUM = ABS(XYZMAX(N)-XYZMIN(N))
IF(YDUM.GT.DMAX) DMAX = YDUM
5 CONTINUE
PSCALE = DMAX/ PLOTSZ
DO 10 L=1,2
DO 10 M=1,2
DO 10 N=1,2
X = XYZMIN(1)
IF(L.EQ.2) X = XYZMAX(1)
Y = XYZMIN(2)
IF(M.EQ.2) Y = XYZMAX(2)
Z = XYZMIN(3)
IF(N.EQ.2) Z = XYZMAX(3)
XROT = A(I,1)*X+A(I,2)*Y+A(I,3)*Z
YROT = A(J,1)*X+A(J,2)*Y+A(J,3)*Z
IF(L*M*N.NE.1) GO TO 30
CONTINUE
20 XRMIN = XROT
XRMAX = XROT
YRMIN = YROT
YRMAX = YROT
30 CONTINUE
IF(XROT.LT.XRMIN) XRMAX = XROT
IF(XROT.GT.XRMAX) XRMIN = XROT
IF(YROT.LT.YRMIN) YRMAX = YROT
IF(YROT.GT.YRMAX) YRMIN = YROT
10 CONTINUE
XR=ABS(XRMAX-XRMIN)
IF(XR/PSCALE.GT.PSIZE) PSCALE=XR/PSIZE
XRMAX = XRMAX/PSCALE
YRMAX = YRMAX/PSCALE
XRMIN = XRMIN/PSCALE
YRMIN = YRMIN/PSCALE
PL0T3720
PL0T3730
PL0T3740
PL0T3750
PL0T3760
PL0T3770
PL0T3780
PL0T3790
PL0T3800
PL0T3810
PL0T3820
PL0T3830
PL0T3840
PL0T3850
PL0T3860
PL0T3870
PL0T3880
PL0T3890
PL0T3900
PL0T3910
PL0T3920
PL0T3930
PL0T3940
PL0T3950
PL0T3960
PL0T3970
PL0T3980
PL0T3990
PL0T4000
PL0T4010
PL0T4020
PL0T4030
PL0T4040
PL0T4050
PL0T4060
PL0T4070
PL0T4080
PL0T4090
PL0T4100
PL0T4110
PL0T4120
PL0T4130
PL0T4140
PL0T4150
PL0T4160
PL0T4170
PL0T4180
PL0T4190
PL0T4200

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```

DELX = -XRMIN
DELY = -YRMIN
XRGN = (PSIZE-XR/PSCALE)/2.0
RETURN
END
SUBROUTINE XPLDD(NEND,X,Y,Z,NODE)
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C *** FOR GENERATING EXPLDED PLOTS.
C *** CALLED BY PLOTX
C
COMMON/CONTRL/ KGEOM,KDATA,KSYMXY,KSYMXYZ,KNOTAT,XLHT,
1KHORZ,KVERT,PHI,THETA,PSI,NEFR,ISCALE,PNTSZ,XORGN,YORGN,
2PSCALE,KDISP,DWAG,KODE
DIMENSION X(20),Y(20),Z(20),NODE(20)
C *** TO CALCULATE THE INCENTER OF TRIANGLES
C
IF(NODE(4).EQ.0) NEND=3
IF(NEND.NE.3) 30 TO 20
10 CONTINUE
A = SQRT((X(2)-X(3))*#2+(Y(2)-Y(3))*#2+(Z(2)-Z(3))*#2)
B = SQRT((X(1)-X(3))*#2+(Y(1)-Y(3))*#2+(Z(1)-Z(3))*#2)
C = SQRT((X(1)-X(2))*#2+(Y(1)-Y(2))*#2+(Z(1)-Z(2))*#2)
AC1 = A/(A+B+C)
AC2 = B/(A+B+C)
AC3 = C/(A+B+C)
XOC = AC1*X(1)+AC2*X(2)+AC3*X(3)
YOC = AC1*Y(1)+AC2*Y(2)+AC3*Y(3)
ZOC = AC1*Z(1)+AC2*Z(2)+AC3*Z(3)
GO TO 190
20 CONTINUE
C *** TO CALCULATE THE CENTROID OF RODS, BARS, AND QUADS
C
XOC = 0.0
YOC = 0.0
ZOC = 0.0
FND=0.0
DO 100 I=1,NEND
IF(NODE(I).EQ.0) GO TO 100
IF(I.GT.8) GO TO 101
FND=FND+1.0
XOC = XOC+FND
100 PL0T4210
PL0T4220
PL0T4230
PL0T4240
PL0T4250
PL0T4260
PL0T4270
PL0T4280
PL0T4290
PL0T4300
PL0T4310
PL0T4320
PL0T4330
PL0T4340
PL0T4350
PL0T4360
PL0T4370
PL0T4380
PL0T4390
PL0T4400
PL0T4410
PL0T4420
PL0T4430
PL0T4440
PL0T4450
PL0T4460
PL0T4470
PL0T4480
PL0T4490
PL0T4500
PL0T4510
PL0T4520
PL0T4530
PL0T4540
PL0T4550
PL0T4560
PL0T4570
PL0T4580
PL0T4590
PL0T4600
PL0T4610
PL0T4620
PL0T4630
PL0T4640
PL0T4650
PL0T4660
PL0T4670
PL0T4680

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YOC = YOC+Y(1)
ZOC = ZOC+Z(1)
100 CONTINUE
101 XOC=XOC/FNO
YOC=YOC/FNO
ZOC=ZOC/FNO
190 CONTINUE
C *** TO REDUCE THE SIZE OF THE ELEMENT
C
DO 200 I=1,NEND
IF(NODE(I) .EQ. 0) GO TO 200
X(I) = X(I)*DMAG
Y(I) = Y(I)*DMAG
Z(I) = Z(I)*DMAG
200 CONTINUE
C *** TO CALCULATE THE CENTROID OF THE REDUCED ELEMENT
C
XRC = XOC*DMAG
YRC = YOC*DMAG
ZRC = ZOC*DMAG
C *** SHIFT CORNERS OF ORIGINAL AND REDUCED TO MAKE CENTROIDS MATCH
C
DO 400 I=1,NEND
IF(NODE(I) .EQ. 0) GO TO 400
X(I) = X(I)+((XOC-XRC)
Y(I) = Y(I)+((YOC-YRC)
Z(I) = Z(I)+((ZOC-ZRC)
400 CONTINUE
RETURN
END
SUBROUTINE GARRROW(X1,Y1,X2,Y2,NC,XHEAD,YHEAD)
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C *** TO DRAW ARROWS FROM X1,Y1 TO X2,Y2.
C *** CALLED BY PLOTX
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C
DEN = SQRT((X2-X1)**2+(Y2-Y1)**2)
IF(DEN .EQ. 0) GO TO 5000
C = (X1-X2)/DEN
S = (Y1-Y2)/DEN
CALL CALPLT(X1,Y1,3)
PLOT4690
PLOT4700
PLOT4710
PLOT4720
PLOT4730
PLOT4740
PLOT4750
PLOT4760
PLOT4770
PLOT4780
PLOT4790
PLOT4800
PLOT4810
PLOT4820
PLOT4830
PLOT4840
PLOT4850
PLOT4860
PLOT4870
PLOT4880
PLOT4890
PLOT4900
PLOT4910
PLOT4920
PLOT4930
PLOT4940
PLOT4950
PLOT4960
PLOT4970
PLOT4980
PLOT4990
PLOT5000
PLOT5010
PLOT5020
PLOT5030
PLOT5040
PLOT5050
PLOT5060
PLOT5070
PLOT5080
PLOT5090
PLOT5100
PLOT5110
PLOT5120
PLOT5130
PLOT5140
PLOT5150
APR1981

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APR1981
PL0T5180
PL0T5190
PL0T5200
APR1981
PL0T5220
PL0T5230
PL0T5240
APR1981
PL0T5260
APR1981
PL0T5280
PL0T5290
PL0T5300
APR1981
PL0T5320
PL0T5330
PL0T5340
APR1981
PL0T5360
APR1981
PL0T5380
PL0T5390
PL0T5400
PL0T5410
PL0T5420
PL0T5430
PL0T5440
PL0T5450
PL0T5460
PL0T5470
PL0T5480
PL0T5490
PL0T5500
PL0T5510
PL0T5520
PL0T5530
PL0T5540
PL0T5550
PL0T5560
PL0T5570
PL0T5580
PL0T5590
PL0T5600
PL0T5610
PL0T5620
PL0T5630
PL0T5640
PL0T5650

CALL CALPLT(X2,Y2,2)
IF(NC.LT.1)GOTO1000
XA=X2+(C*XHEAD-C*YHEAD)
YA=Y2+(S*XHEAD+C*YHEAD)
CALL CALPLT(XA,YA,2)
IF(NC.LT.2)GOTO1000
XB=X2+(C*XHEAD-S*(-YHEAD))
YB=Y2+(S*XHEAD+C*(-YHEAD))
CALL CALPLT(XB,YB,2)
CALL CALPLT(XB,YB,2)
CALL CALPLT(X2,Y2,2)
IF(NC.LT.4)GOTO1000
XC=X2+(-S*YHEAD)
YC=Y2+(+C*YHEAD)
CALL CALPLT(XC,YC,2)
IF(NC.LT.5)GOTO1000
XD=X2+(-S*(-YHEAD))
YD=Y2+(+C*(-YHEAD))
CALL CALPLT(XD,YD,2)
1000 CONTINUE
CONTINUE CALPLT(X2,Y2,3)
5000 RETURN
END
SUBROUTINE NDLET(NUMPT,XPT,YPT,ZPT,UPT,VPT,WPT)
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C *** FOR ANNOTATING GRID POINT NUMBERS ON PLOTS.
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
COMMON/CONTROL/KGEOM,KDATA,KPLOT,KSYMXY,KSYMZX,NOTAT,XLHT,
1KHORZ,KVERT,KTHETA,PSI,NEWFR,ISCALE,PLOTSZ,XORGN,YORGN,
2PSCALE,KDISP,DAG,KODE
COMMON/LIMITS/XMAX,YMAX,ZZMAX,XXMIN,YYMIN,ZZMIN,NDMIN,
1NELMAX,NELMIN
COMMON/XYZLIM/XYZMAX(3),XYZMIN(3)
COMMON/ABLK/A(3,3)
COMMON/ABLK/ABLK
COMMON/ABLK/ABLK
COMMON/ABLK/ABLK
COMMON/ABLK/ABLK
COMMON/ABLK/ABLK
COMMON/ABLK/ABLK
COMMON/PDELS/DELX,DELY
DIMENSION NUMPT(1),XPT(1),YPT(1),ZPT(1),UPT(1),VPT(1),WPT(1)
II=KHORZ
JJ=KVERT
XSHIFT=0.0
YSHIFT=0.0
ZSHIFT=0.0

```

```

      PLOT5650
      PLOT5660
      PLOT5670
      PLOT5680
      PLOT5690
      PLOT5700
      PLOT5710
      PLOT5720
      PLOT5730
      PLOT5740
      PLOT5750
      PLOT5760
      PLOT5770
      PLOT5780
      PLOT5790
      PLOT5800
      PLOT5810
      PLOT5820
      PLOT5830
      PLOT5840
      PLOT5850
      PLOT5860
      PLOT5870

DO 500 I=1,NNJDE
  IF(NUMP(I).LE.0) GO TO 500
  IF(NUMP(I).LT.NDMIN.OR.NUMP(I).GT.NDMAX) GO TO 500
  IF(XPT(I).LT.GT.XYZMAX(1)) GO TO 500
  IF(XPT(I).LT.GT.XYZMIN(1)) GO TO 500
  IF(XPT(I).LT.GT.XYZMAX(2)) GO TO 500
  IF(XPT(I).LT.GT.XYZMIN(2)) GO TO 500
  IF(XPT(I).LT.GT.XYZMAX(3)) GO TO 500
  IF(XPT(I).LT.GT.XYZMIN(3)) GO TO 500
  IF(YPT(I).LT.XSHIFT1/PSCALE) YPT(I)+YSHIFT1/PSCALE
  IF(ZPT(I).LT.XSHIFT1/PSCALE) ZPT(I)+ZSHIFT1/PSCALE
  XY = (ZPT(I)+ZSHIFT1/PSCALE-A(I,I)*X+A(I,J,1)*X+A(I,J,2)*Y+A(I,J,3)*Z
  XROT = A(I,J,1)*X+A(I,J,2)*Y+A(I,J,3)*Z
  YROT = XROT+XLHT/2.0
  XL = XROT+XLHT/2.0
  YL = YROT+XLHT/2.0
  XL = XL+DELX
  YL = YL+DELY
  AL = NUMPT(I)
  CALL CALNUM(XL,YL,XLHT,AL,J,0,-1)
  RETURN
END
500 CONTINUE

```

```

SUBROUTINE INITIAL
C * * * * *
C *** TO SET UP VALUES FOR CONTROL PARAMETERS
C * * * * *
COMMON/CDATA/NTIME,NITLC
COMMON/CONTRL/KGEOM,KDATA,KPLDT,KSYMXY,KSYMXXZ,KNOTAT,XLHT,
1KHORZ,VERT,PHI,THETA,PSI,NEWFR,ISCALF,PLOTSZ,XORGN,YORGN,
2PSCALE,KDISP,DMAKODE
COMMON/LIMITS/XMAX,YMAX,ZMAX,YYMIN,ZZMIN,NDMAX,NDMIN,
1NELMAX,NELMIN,NELMAX,NELMIN,NCOMMON/CORGN/YMAX,YSPACE,PSIZE
COMMON/SAVEV/DMASS,IMAS
COMMON/KOUNT/VNODE,NNDEST,NUDISP,NWDISP,NWDISP
COMMON/SEQNCE/IRESEQ
COMMON/VALUFS/NVALUFS
COMMON/CASE/ID,ICASE
NAMELIST/OPTION/
1KGEM,KDATA,NVALUS,IRESEQ,KPLOT,VSPACE,PSIZE,ICASE
C *** DESCRIPTION OF VALUES IN &PTION GIVEN IN SUBROUTINE DJCANT
C *** TO SET DEFAULT VALUFS FOR &OPTION
C
C NNDEST = 200
C NUDISP=0
C NWDISP=0
C KGFOM=9
C KDATA=9
C NTIME=0
C NVALUS = 0
C IRESEQ = 1
C KPLOT = 1
C YSPACE=2.0
C PSIZE=9.0
C ICASE = 0
C *** TO SET DEFAULT VALUES FOR EPICT
C KHORZ = 1

```



```

COMMON/CONTRP/ KGEOM, KDAIA, KPLOT, KSYMXY, KSYMXX, KSYMZZ, NOTAT, XLHT,
1 KHORZ, KVERT, PHI, THETA, PSI, NEWFR, ISCALE, PLOTSZ, XJRGN, YURGN,
2 PSCALE, KDISPS, DVAG, KODE
COMMON/LIMITS/ XXXMAX, YMAX, ZMAX, XXMIN, YYMIN, ZZMIN, NDMAX, NDMIN,
1 NELMAX, NELMIN
COMMON/XYZLIM/ XYZMAX(3), XYZMIN(3)
COMMON/KOUNT/ NNODE, NNODEST, NUDISP, NVUDISP, NUMPT(1), ZPT(1), VPT(1), WPT(1)
DIMENSION NOD(20)
DO 5 I=1,3
XYZMIN(I) = +1.0E20
XYZMAX(I) = -1.0E20
5 CONTINUE
REWIND 10
100 CONTINUE
IF(NUMEL.EQ.1) END=NEND
1 IF(NUMEL.LT.NELMIN.OR.NUMEL.GT.NELMAX) GO TO 100
DO 10 I=1,NEND
ND = NODE(I)
IF(NODE(I).EQ.2) GO TO 10
IF{NUMPT(ND).LT.NDMIN.OR.NJMP(ND).GT.NDMAX} GO TO 100
10 CONTINUE
DO 20 I=1,NEND
IF(NODE(I).EQ.1) GO TO 20
ND = NODE(I)
IF(XPT(ND).GT.XXMAX) GO TO 20
IF(XPT(ND).LT.XXMIN) GO TO 20
IF(YPT(ND).GT.YYMAX) GO TO 20
IF(YPT(ND).LT.YYMIN) GO TO 20
IF(ZPT(ND).GT.ZZMAX) GO TO 20
IF(ZPT(ND).LT.ZZMIN) GO TO 20
IF(XPT(ND).GT.XYZMAX(1)) XYZMAX(1) = XPT(ND)
IF(XPT(ND).LT.XYZMIN(1)) XYZMIN(1) = XPT(ND)
IF(YPT(ND).GT.XYZMAX(2)) XYZMAX(2) = YPT(ND)
IF(YPT(ND).LT.XYZMIN(2)) XYZMIN(2) = YPT(ND)
IF(ZPT(ND).GT.XYZMAX(3)) XYZMAX(3) = ZPT(ND)
IF(ZPT(ND).LT.XYZMIN(3)) XYZMIN(3) = ZPT(ND)
20 CONTINUE
GO TO 100
1000 CONTINUE
DO 300 I=1,3
IF(I.EQ.1.AND.KSYMYZ.NE.1) CO TO 300
IF(I.EQ.2.AND.KSYMXX.NE.1) GO TO 300
IF(I.EQ.3.AND.KSYMXY.NE.1) GO TO 300
XYZBIG = ABS(XYZMAX(1))
IF(ABS(XYZMIN(1)).GT.XYZBIG) XYZBIG = ABS(XYZMIN(1))
XYZMAX(I) = XYZBIG
XYZMIN(I) = -XYZBIG

```

```

300 CONTINUE
      RETURN
    END
    SUBROUTINE ZERJD(NUVPT,XPT,YPT,ZPT,UPT,VPT,WPT)
C *   *   *   *   *   *   *   *   *   *   *   *   *   *   *
C *** INITIALIZES ALL DISPLACEMENTS TO ZERO.
C *   *   *   *   *   *   *   *   *   *   *   *   *   *   *
C COMMON/KOUNT/ NNODE,NNDEST,NUDISP,NWDISP
C DIMENSION NUMPT(1),XPT(1),YPT(1),ZPT(1),UPT(1),VPT(1),WPT(1)
C IF(NUDISP.EQ.0) GO TO 200
C DO 150 I=1,NUDISP
C     UPT(I)=0.0
C 150 CONTINUE
C 200 CONTINUE
C IF(NWDISP.EQ.0) GO TO 300
C DO 250 I=1,NWDISP
C     VPT(I)=0.0
C 250 CONTINUE
C 300 CONTINUE
C IF(NWDISP.EQ.0) GO TO 400
C DO 350 I=1,NWDISP
C     WPT(I)=0.0
C 350 CONTINUE
C 400 RETURN
    END
    SUBROUTINE PNTJUT(OUT,NJAPT,XPT,YPT,ZPT,UPT,VPT,WPT)
C *   *   *   *   *   *   *   *   *   *   *   *   *   *   *
C *** FOR PRINTED OUTPUT OF INFORMATION IN BLANK COMMON - ZZZ
C *   *   *   *   *   *   *   *   *   *   *   *   *   *   *
C COMMON/KOUNT/ NNODE,NNDEST,NUDISP,NWDISP
C DIMENSION NUMPT(1),XPT(1),YPT(1),ZPT(1),UPT(1),VPT(1),WPT(1)
C DIMENSION NODE(20)
C GO TO (1000,2500), IOUT
C 1000 CONTINUE
C *** FOR OUTPUT OF GEOMETRY INFORMATION

```

```

16 WRITE(6,16)
17 FORMAT(6,17)
18 FORMAT(5X,'RESEQUENCED',4X,'USER INPUT',/
      15X,'GRID POINT',5X,'GRID POINT',/
      25X,'NUMBER',9X,'NUMBER',13X,'X',14X,'Y',14X,'Z',//)
DO 30 I=1,NODE
  WRITE(6,18) 1,NUMPT(I),XPT(I),VPT(I)
  WRITE(6,18) 1,NUMPT(I),XPT(I),ZPT(I)
19 FORMAT(5X,'ELEMENT INFORMATION - WITH RESEQUENCED GRID POINTS',/
      1,//)
  WRITE(6,9008)
9008 FORMAT(1X,'RESEQUENCED',4X,'USER INPUT',25X,'GRID POINTS',/
      11X,'ELEMENT',8X,'ELEMENT',/
      21X,'NUMBER',9X,'NUMBER',7X,'NUMBER',12,13,14,15,16,17,18,19,20,/,1
      3,8,9,10,11,12,13,14,15,16,17,18,19,20,/,1
      REWIND 10
      I=0
35 CONTINUE
I=I+1
READ(10,END=999) NEND,NUMEL,(NODE(J),J=1,NEND)
IF(NEND.EQ.12) GO TO 40
WRITE(6,9010) 1,NUMEL,(NODE(J),J=1,NEND)
9010 FORMAT(1X,14,11X,14,9X,20,/)
GO TO 35
40 WRITE(6,9010) 1,NUMEL,(NODE(J),J=1,4),(NODE(J),J=9,12)
GO TO 35
2000 CONTINUE
C *** FOR OUTPUT OF DISPLACEMENT DATA
C
210 FORMAT(6,210)
  WRITE(6,17)
  DO 230 I=1,NNODE
    U=0.0
    IF(NUDISP.NE.0) U = UPT(I)
    V=0.0
    IF(NDISP.NE.0) V = VPT(I)
    W=0.0
    IF(NWDISP.NE.0) W = WPT(I)
    WRITE(6,18) I,NUMPT(I),U,V,W
230 CONTINUE
999 RETURN
END
SUBROUTINE ELTYPE(MTYPE,GEOM)

```


6 CONTINUE
6 WRITE(6,9006)
9006 FORMAT(1,1X,A3)NORMAL TERMINATION IN S3L21,ELEMENT CARD ERROR•/1
GO TO 1000
7 CONTINUE
7 WRITE(6,9007)
FORMAT(1,1X,A3)NORMAL TERMINATION IN ADTRUS,ELEMENT CARD ERROR•/1
GO TO 1000
8 CONTINUE
8 WRITE(6,9008)
FORMAT(1,1X,A3)NORMAL TERMINATION IN ADPLAN,ELEMENT CARD ERROR•/1
GO TO 1000
9 CONTINUE
9 WRITE(6,9009)
FORMAT(1,1X,A3)NORMAL TERMINATION IN AD3DEE,ELEMENT CARD ERROR•/1
GO TO 1000
10 CONTINUE
10 WRITE(6,9010)
FORMAT(1,1X,A3)NORMAL TERMINATION IN AD8EAM,ELEMENT CARD ERROR•/1
GO TO 1000
11 CONTINUE
11 WRITE(6,9011)
FORMAT(1,1X,A3)NORMAL TERMINATION IN NSTRUS,ELEMENT CARD ERROR•/1
GO TO 1000
12 CONTINUE
12 WRITE(6,9012)
FORMAT(1,1X,A3)NORMAL TERMINATION IN NSPLAN,ELEMENT CARD ERROR•/1
GO TO 1000
13 CONTINUE
13 WRITE(6,9013)
FORMAT(1,1X,A3)NORMAL TERMINATION IN NS3DEE,ELEMENT CARD ERROR•/1
GO TO 1000
14 CONTINUE
14 WRITE(6,9014)
FORMAT(1,1X,A3)NORMAL TERMINATION NONSAP MESH CANNOT BE PLOTTED•/1
GO TO 1000
15 CONTINUE
15 GO TO 1000
16 CONTINUE
16 GO TO 1000
17 CONTINUE
17 GO TO 1000
18 CONTINUE
18 GO TO 1000
19 CONTINUE
19 GO TO 1000
20 CONTINUE

```

1000 CONTINUE
CALL PSTOP
RETURN
END
SUBROUTINE GEOM9( NUMPT,XPT,YPT,ZPT,UPT,VPT,WPT)
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
*** GEOM9 READS SAP IV GEOMETRY DATA
*** CALLED BY PSAPI
C
C COMMON/CONTRL/ KGEOM1,KDATA,KPLOT,KSYMXY,KSYMZ,NOTAT,XLHT,
1KHORZ,KVERT,PHI,THETA,PSI,NEWFR,ISCALE,PLOT SZ,XORGN,YORGN,
2PSCALE,KDISP,DAG,KODE
COMMON/KOUNT/NODE,NODE,NODE,NODE,NODE,NODE,NODE,NODE,NODE,NODE
COMMON/GCONT/NJ,MNP,NPAR{20},NELTY,PNJMEL
DIMENSION NUMPT(1),XPT(1),YPT(1),ZPT(1),UPT(1),VPT(1),WPT(1)
DATA CTEST/./
C
C *** INSERT ROUTINE HERE
C
C READ(5,100) HED
100 FORMAT(12A6)
C
C
C *** READ MASTER CONTROL CARD
*** NUMNP = TOTAL NUMBER OF NODE POINTS
*** NELTYP = NUMBER OF ELEMENT GROUPS
C
C 200 READ(5,200) NUMNP, NELTYP
200 FORMAT(2I5)
NNODE=NUMNP
C
C*****READ OR GENERATE NODAL POINT DATA
C
C NOLD=0
10 READ(5,9006) CT,N,XPT(N),YPT(N),ZPT(N),KN
9006 FORMAT(1I14,3O10.0,15)
C
C ***CHECK FOR CYLINDRICAL COORDINATES
C
IF(CT.NE.CTEST) GO TO 20
R=XPT(N)
XPT(N)=R*R*SIN(ZPT(N)/57.2958)
ZPT(N)=R*COS(ZPT(N)/57.2958)

```

```

20 CONTINUE
NUMPT(N)=N
IF (NOLD.EQ.0) GO TO 50
C*****CHECK IF GENERATION IS REQUIRED
      IF ((KN.EQ.0) GO TO 50
      NUM=(N-NOLD)/KN
      NUMN=NUM-1
      IF (NUMN.LT.1) GO TO 50
      XNUM=NUM
      DX=(XPT(N)-XPT(NOLD))/XNUM
      DY=(YPT(N)-YPT(NOLD))/XNUM
      DZ=(ZPT(N)-ZPT(NOLD))/XNUM
      K=NOLD
      DO 30 J=1,NUMN
      KK=K
      K=K+KN
      XPT(K)=XPT(KK)+DX
      YPT(K)=YPT(KK)+DY
      ZPT(K)=ZPT(KK)+DZ
      NUMPT(K)=K
      30 CONTINUE
      NOLD=N
      IF (N.NE.NUMNP) GO TO 10
      NUMEL=0
C***** READ ELEMENT CONTROL CARDS
      DO 900 M=1,NELTYP
      READ(51001,1EN) =9991 (NPARI), I=1,14)
      1001 WRITE(6,9010) (NPARI,I=1,14)
      9010 FORMAT(1H,/1. YPAR = 1,2015//1,
      MTYPE=NPARI)
      CALL ELTYPE(MTYPE,KGEOM)
      900 CONTINUE
      ENDFILE 10
      999 RETURN
      END
      SUBROUTINE TRSS
C * * * * * READ SAP IV TRUSS ELEMENT CARDS (ELTYPE 1)
C *** CALLED BY ELTYPE * * * * *
C COMMON/GCONT/NUMNP,NPAR(20),NELTYP,NUMEL

```

```

N2=2
NUMMAT=NPAR(2)
NUMMAT=NPAR(3)
C *** READ MATERIAL PROPERTY CARDS (DUMMY)
DO 10 I=1,NUMMAT
  READ(5,1001) DUMMY
1001 FORMAT(10A8)
10 CONTINUE
C *** READ ELEMENT LOAD MUL. (DUMMY)
DO 20 I=1,4
  READ(5,1001) DUMMY
20 CONTINUE
IF(NPAR(14).EQ.0) NPAR(14)=1
N=NPAR(14)
C *** READ ELEMENT CONNECTION INFORMATION OR GENERATE
  READ(5,1004) I,J,NTYP,TEH,KK
1004 FORMAT(14I5,F10.0,15)
IF(KK.EQ.0) KK=1
120 IF(I.NE.N) GO TO 200
I=II
J=JJ
KK=KK
C
200 CONTINUE
NUMEL=NUMEL+1
WRITE(10) N2,N,I,J
IF(N.EQ.NUMEL) RETURN
N=N+1
I=I+KKK
J=J+KKK
IF(N.GT.M) GO TO 100
GO TO 120
END
SUBROUTINE PLANE
C * * * * *
C *** READS SAP IV MEMBRANE ELEMENT CARDS (ELTYPE 3)
*** CALLED BY ELTYPE
C * * * * *
C DIMENSION EMJI(4,5),IE(5),IX(4)
COMMON/GCON/ NNP,NPAR(20),NELTYP,NUMEL
N4=4
NUME=NPAR(2)
NUMMAT=NPAR(3)
C *** READ MATERIAL PROPERTIES
DO 60 M=1,NUMMAT

```

```

1010 READ(5,1010) MAT,NT
      FORMAT(2,5) NT=1
      NTC=2*NT
      DO 50 K=1,NTC
      READ(5,1005) DUMMY
1005 FORMAT(10A8)
      50 CONTINUE
      60 CONTINUE

C*** READ ELEMENT LOAD FACTORS
C   READ(5,1002) {(EMUL(I,J),J=1,5),I=1,4}
1002 FORMAT(5F10.0)

C*** READ ELEMENT PROPERTIES
C   IF(NPAR(14).EQ.0) NPAR(14)=1
      N=NPAR(14)-1
1003 READ(5,1003) M,{IE(I),I=1,4},KG
      FORMAT(5I5) 30X,I5
      IF(KG.EQ.0) KG=1
140  N=N+1
      IF(N.EQ.N) GO TO 145
      DO 142 I=1,4
      IX(I)=IX(I)+KG
142  GO TO 150
145  DO 148 I=1,4
148  IX(I)=IE(I)
150  CONTINUE
      I = IX(1)
      J = IX(2)
      K = IX(3)
      L = IX(4)
      NUMEL=NUMEL+1
      WRITE(10,N4,N1,IJK,L)
310  IF(N.EQ.NUMEL) RETURN
      IF(N.EQ.M) GO TO 130
      GO TO 140
END

SUBROUTINE BEAM
C * * * * *
C *** READS SAP IV BEAM ELEMENT CARDS (ELTYPE 2)
C *** CALLED BY ELTYPE
C * * * * *

```

```

C COMMON/GCONT/VNUMNP,NPAR(20),NLTYP,NUMEL
N2=2
NUMEPC=NPAR(2)
NUMFEF=NPAR(3) * 2
NUMMAT=NPAR(5)
SAPF1890
SAPF1900
SAPF1910
SAPF1920
SAPF1930
SAPF1940
SAPF1950
SAPF1960
SAPF1970
SAPF1980
SAPF1990
SAPF2000
SAPF2010
SAPF2020
SAPF2030
SAPF2040
SAPF2050
SAPF2060
SAPF2070
SAPF2080
SAPF2100
SAPF2110
SAPF2120
SAPF2130
SAPF2140
SAPF2150
SAPF2160
SAPF2170
SAPF2180
SAPF2190
SAPF2200
SAPF2210
SAPF2220
SAPF2230
SAPF2240
SAPF2250
SAPF2260
SAPF2270
SAPF2280
SAPF2290
SAPF2300
SAPF2310
SAPF2320
SAPF2330
SAPF2340
SAPF2350
SAPF2360
C *** READ MATERIAL PROPERTY CARDS (DUMMY)
      DO 10 I=1,NUMAT
      READ(5,1001) DUMMAY
1001 FORMAT(10A8)
10 CONTINUE
C READ ELEMENT PROPERTY CARDS (DUMMY1)
      DO 20 J=1,NUMEP
      READ(5,1001) DUMMYY1
20 CONTINUE
C READ ELEMENT LOAD MULTIPLIERS(DUMMY2)
      DO 30 K=1,3
      READ(5,1001) DUMMYY2
30 CONTINUE
C READ FIXED-END FORCE CARDS(DUMMY3)
      DO 40 L=1,NUMEFF
      READ(5,1001) DUMMYY3
40 CONTINUE
40 IF(NPAR(14).EQ.0) NPAR(14)=1
N=NPAR(14)
C *** READ ELEMENT CONNECTION INFO
      DO 100 READ(5,1002) I,I,JJ,KK
1002 FORMAT(3I5) I,JJ,KK=1
100 IF(I.EQ.N) GO TO 200
      I = I
      J = JJ
      KKK = KK
200 CONTINUE
      NUMEL = NUMEL+1
      WRITE(10,N2,N,1,J)
      IF(N.EQ.NUMEL) RETURN
      N = N+1
      I = I + KKK
      J = J + KKK
      IF (N.GT.M) GO TO 100
      GO TO 120
END
SUBROUTINE THREED
C * * * * *
C

```

```

C *** THIS SUBROUTINE READS SAP IV 3-D 8 NODE BRICK ELEMENTS
C *** CALLED BY ELTTYPE
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C DIMENSION INP(8),NP(8)
COMMON/GCON/NMNP,NPAR(20),NLTYP,NJNEL
N8=8
      NUME=NPAR(2)
      NUMMAT=NPAR(3)
      NDISLD=NPAR(4)
      READ THE MATERIAL PROPERTIES
      DO 50 M=1,NUMMAT
           READ(5,9002) DUMMY
      9002 FORMAT(20A4)
CONTINUE
C *** READ DISTRIBUTED SURFACE LOADS
      IF(NDISLD.EQ.0) GO TO 61
      DO 60 M=1,NDISLD
           READ(5,9002) DUMMY
      60 CONTINUE
      61 CONTINUE
      62 READ(ACCELERATION DUE TO GRAVITY
      63 READ(5,9002) DUMMY
C *** READ ELEMENT LOAD CASE MULTIPLIERS
      DO 80 I=1,5
           READ(5,9002) DUMMY
      80 CONTINUE
      81 IF(NPAR(14).EQ.0) NPAR(14)=1
      NEL=NPAR(14)-1
      130 READ(5,9006) INEL,(INP(I),I=1,8),ININT,IMAT,IINC
      9006 FORMAT(12I5)
           IF(IINC.EQ.0) IINC=1
      140 NEL=NEL+1
           ML=INEL-NEL
           IF(ML)150,155,160
      150 CALL ERROR(5)
           NO GENERATION OF NODE POINTS REQUIRED
      C 155 DO 156 I=1,8
           NP(I)=INP(I)
      156 CONTINUE
           GO TO 162
C *** GENERATION REQUIRED
      C 160 DO 161 I=1,8
           NP(I)=NP(I)+IINC
      161 CONTINUE
           NUMEL=NUMEL+1

```

```

      WRITE(11) N8,NEL,(NP(I),I=1,8)
      IF(NEL.EQ.NUME) RETURN
      IF(NEL.EQ.INEL) GO TO 130
      GO TO 140
      END

      SUBROUTINE SHELL
      C *   *   *   *   *   *   *   *   *   *   *   *   *   *
      C *** READS SAP IV SHELL ELEMENT CARDS (ELTYPE 6)
      C *** CALLED BY ELTYPE
      C *   *   *   *   *   *   *   *   *   *   *   *   *   *
      C
      C DIMENSION LY(7),IX(4)
      COMMON/GCONT/NUMNP,NPAR(20),NELTYP,NJTEL
      N4=4
      ISTOP=0
      NUME=NPAR(12)
      NUMMAT=NPAR(3)
      NMAT=2*NUMMAT
      C *** READ MATERIAL PROPERTIES (DUMMY)
      DO 10 N=1,NMAT
      READ(5,1000) DUMMY
      1000 FORMAT(10A8)
      10 CONTINUE
      C *** READ ELEMENT LOAD FACTORS (DUMMY)
      20 READ(5,200) K=1,5
      CONTINUE
      IF(NPAR(14).EQ.0) NPAK(14)=1
      NN=NPAR(14)-1
      READ(5,1001) MM,IY
      100 READ(5,1001) MM,IY
      1001 FORMAT(18I5)
      110 NN=NN+1
      IF(MM-NN) 440,50,60
      50 DO 45 I=1,7
      45 IX(I)=IY(I)
      INCL=IY(7)
      IF(INCL-EQ.0) INCL=1
      GO TO 70
      DO 65 I=1,4
      65 IX(I)=IX(I)+INCL
      70 I=IX(1)
      J=IX(2)
      K=IX(3)

```

```

L=IX(4)      NUMEL = NUMEL +1
WRITE(101,N4,NV,I,J,K,L
      GOTO 500
      FORMAT(19HOCARD FOR ELEMENT(,15,14H) IS IN ERROR.,1X)
440   ISTOP = 1
      IF(NN.LT.MM) GO TO 110
      IF(ISTOP.EQ.1) STOP
      GO TO 100
      END
      SUBROUTINE BNJRY
C *   *   *   *   *   *   *   *   *   *   *   *   *   *   *
C *** READS SAP IV BOUNDARY ELEMENT CARDS (ELTYPE 7)
C *** BOUNDARY ELEMENTS ARE NOT PLOTTED
C *** CALLED BY ELTYPE
C *   *   *   *   *   *   *   *   *   *   *   *   *   *   *
C COMMON/GCONT/NUMNP,NPAR(20),NELTYP,NUMEL
      NUME=NPAR(2)
      READ(5,1002) DUMMY
      READ(5,1002) DUMMY
      1002 FORMAT(10A8)
      READ BOUNDARY ELEMENT CARDS
      N=0
      100  READ(5,1004) M,II,KK
      N=N+1
      IF(M.GE.NUME) RETURN
      IF(KK.GT.0) GOTO 200
      GOTO 100
      READ(5,1004) M2,II2,KK2
      N=N+(M2-M)/KK
      IF(M.GE.NUME) RETURN
      GOTO 100
      1004 FORMAT(2I5,25X,15)
      END
      SUBROUTINE SOL21
C *   *   *   *   *   *   *   *   *   *   *   *   *   *   *
C *** THIS SUBROUTINE READS SAP IV 3-D, 8-20 NODE BRICK ELEMENTS
C *** CALLED BY ELTYPE
C *   *   *   *   *   *   *   *   *   *   *   *   *   *   *

```

```

C DIMENSION NP(20),INP(20),NJMN,NPAR(20),NELTYP,NUMEL
COMMON/GCONT/NJMN,NP,INP(20)
N20=20
NSOL21=NPAR(2)
NUMMAT=NPAR(3)
MAXTP=NPAR(4)
IF(MAXTP.EQ.0) MAXTP=1
NORTH0=NPAR(5)
NDLS=NPAR(6)
MAXNOD=NPAR(7)
IF(MAXNOD.EQ.0) MAXNOD=21
IF(MAXNOD.EQ.8) N20=8
N0PSET=NPAR(8)
READ THE MATERIAL PROPERTY CARDS
DO 50 J=1,NUMMAT
  READ(5,9002) M,NTP
  FORMAT(215) M,NTP
  IF(NTP.EQ.0) NTP=1
  NTP2=2*NTP
  DO 40 JJ=1,NTP2
    READ(5,9004) DUMMY
    9004 FORMAT(20A4)
    40 CONTINUE
    50 CONTINUE
C *** READ MATERIAL AXES ORIENTATION SETS
C *** IF(NORTH0.EQ.0) GO TO 61
  DO 60 J=1,NORTH0
    READ(5,9004) DUMMY
    60 CONTINUE
    C 61 CONTINUE
    C *** READ DISTRIBUTED SURFACE LOAD DATA
    IF(NDLS.EQ.0) GO TO 71
    NDLS2=NDLS*2
    DO 70 J=1,NDLS2
      READ(5,9004) DUMMY
    70 CONTINUE
    C 71 CONTINUE
    C *** READ STRESS OUTPUT LOCATION SETS
    IF(NOPSET.EQ.0) GO TO 81
    DO 80 J=1,NOPSET
      READ(5,9004) DUMMY
    80 CONTINUE
    C 81 CONTINUE
    C *** READ ELEMENT LOAD CASE MULTIPLIERS
    DO 90 J=1,5
      READ(5,9004) DUMMY
    90 CONTINUE

```

```

C * ** READ ELEMENT DATA CARDS
      IF(NPAR(14).EQ.0) NPAR(14)=1
      NEL=NPAR(14)-1
130   READ(51,9006) INEL,IINC
9006   FORMAT(15.35X,I5)
      READ(51,9008) INP(I),I=1,N20
9008   FORMAT(16.15)
      IF(IINC.EQ.0) IINC=1
140   NEL=INEL+1
      ML=INEL-NEL
      IF(ML.LT.155) 155,160
150   CALL ERROR(6)
      NO GENERATION OF NODE POINTS REQUIRED
155   DO 156 I=1,N20
      NP(I)=NP(I)
156   GO TO 162
      CONTINUE
C * ** GENERATION OF NODE POINTS REQUIRED
160   DO 161 I=1,N20
      IF(NP(I).EQ.0) GO TO 161
      NP(I)=NP(I)+KN
161   CONTINUE
      NUMEL=NUMEL+1
      WRITE(10) N20,NEL,(NP(I),I=1,N20)
      IF(NEL.EQ.NSOL) RETURN
      IF(NEL.LT.1NEL) GO TO 140
      KN=IINC
      GO TO 130
END
      SUBROUTINE GEOM1(NUPT,XPT,YPT,ZPT,UPT,VPT,WPT)
      C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
      C * * * THIS ROUTINE READS ADINA DATA CARDS FROM THE TITLE CARD TO THE
      C * * * ELEMENT CONTROL CARDS - IT IS CALLED BY PSAPI
      C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
      COMMON/CONTRL/KGEOM,KDATA,KPLOT,KSYMMX,KSYMMZ,KNOTAT,XLHT,
      1KHORZ,KVERT,PHI,KTHETA,PSI,NEWFR,ISCALE,PLOTSZ,XORGN,YORGN,
      2PSCALE,KDISP,DWAG,KODE
      COMMON/KOUNT/NNODE,NDEST,NUDISP,NWDISP
      COMMON/GCOUNT/NUMNP,NPAR(20),NELTYPE,NUMEL
      DIMENSION NUPT(1),YPT(1),ZPT(1),UPT(1),VPT(1),WPT(1),
      1NODE(20),IDDE(6),ID(6),IBOLD(6)
      DATA CTEST/'x'/

```

```

NCARD=0
READ(5,9000) DUMMY
9000 FORMAT(20A4)
C *** READ MASTER CONTROL CARDS
C *** NUMNP = TOTAL NUMBER OF NODE POINTS
C *** NELTYP = NUMBER OF ELEMENT GROUPS
C *** READ(5,9001) NUMNP((IDOF(I),I=1,6),NEGL,MODEX,NSTE
9001 FORMAT(15.6 11.14 315)
      NELTYP=NEGL+NEGNL
      NNODE=NUMNP
      READ(5,9002) IMASS, IDAMP, IMASSN, IDAMPN
9002 FORMAT(415)
      READ(5,9002) IEIG
      READ(5,9002) ISREF, NUMREF, IEQUIT, ITEMAX
      READ(5,9000) DUMMY
      READ(5,9000) DUMMY
      READ(5,9000) DUMMY
      READ(5,9000) DUMMY
      C *** READ OR GENERATE NODAL POINT DATA
      NOLD=0
      NEQ=0
10     READ(5,9006) ICT, N((ID(I),I=1,6), XPT(N), YPT(N), ZPT(N), KN
9006 FORMAT(14,1X,14,1X,14,1X,14,1X,14,1X,14,1X,14,1X,14,1X,14,1X,15)
      C *** CHECK FOR CYLINDRICAL COORDINATES
      IF(CT•NE•CTEST) GO TO 12
      DUM=ZPT(N)/57.2958
      R=YPT(N)
      R=YPT(N)=R*COS(ZPT(N)/57.2958)
      ZPT(N)=R*SIN(ZPT(N)/57.2958)
12     CONTINUE
      NUMP=N
      IF(NOLD.EQ.0) GO TO 50
      C *** FOR GENERATION OF FIXED BOUNDARY CONDITIONS
      DO 15 I=1,6
      IF(IDOLD(I).EQ.-1.AND.ID(I).EQ.0) ID(I)=IDOLD(I)
15     CONTINUE
      IF(KNOLD.EQ.0) GO TO 50
      NUM=(N-NOLD)/KNOLD
      NUM=NUM-1
      IF(NUM.LT.1) GO TO 50
      C *** TO COUNT DOFS TO DETERMINE NUMBER OF IC CARDS
      DO 20 I=1,6
      IF(IDOF(I).EQ.0.AND.IDOLD(I).EQ.0) NEQ=NEQ+NUM
20     CONTINUE
      DX=(XPT(N)-XPT(VOLD))/NUM
      IF(CT•NE•CTEST) GO TO 21
      ROLD=YPT(NOLD)/COS(DUMOLD)
      RNEW=YPT(N)/COS(DUM)
      DR=(RNFW-ROLD)/NUM
      ADNA0190
      ADNA0200
      ADNA0210
      ADNA0220
      ADNA0230
      ADNA0240
      ADNA0250
      ADNA0260
      ADNA0270
      ADNA0280
      ADNA0290
      ADNA0300
      ADNA0310
      ADNA0320
      ADNA0330
      ADNA0340
      ADNA0350
      ADNA0360
      ADNA0370
      ADNA0380
      ADNA0390
      ADNA0400
      ADNA0410
      ADNA0420
      ADNA0430
      ADNA0440
      ADNA0450
      ADNA0460
      ADNA0470
      ADNA0480
      ADNA0490
      ADNA0500
      ADNA0510
      ADNA0520
      ADNA0530
      ADNA0540
      ADNA0550
      ADNA0560
      ADNA0570
      ADNA0580
      ADNA0590
      ADNA0600
      ADNA0610
      ADNA0620
      ADNA0630
      ADNA0640
      ADNA0650
      ADNA0660

```

```

DT=(DUM-DUMOLD)/NUM
GO TO 22
21 CONTINUE
DY=(YPT(N)-YPT(NOLD))/NUM
DZ=(ZPT(N)-ZPT(NOLD))/NUM
22 CONTINUE
K=NOLD
DO 30 J=1,NUMN
  K=K+KNOLD
  XPT(K)=XPT(K)+DX
  IF(CT*NE.CTEST) GO TO 26
  ROLD=ROLD+DR
  DUMOLD=DUMOLD+DT
  YPT(K)=ROLD*COS(DUMOLD)
  ZPT(K)=ROLD*SIN(DUMOLD)
  GO TO 28
26 CONTINUE
  YPT(K)=YPT(K)+DY
  ZPT(K)=ZPT(K)+DZ
28 CONTINUE
  NUMPT(K)=K
  COUNTINUE
30 NOLD=N
  KNOLD=N
  DUMOLD=DUM
C *** TO COUNT DOFS TO DETERMINE NUMBER OF IC CARDS
  DO 55 I=1,6
    IF(IDOF(I).EQ.0.AND.ID(I).EQ.0) NEQ=NEQ+1
    IDOLD(I)=ID(I)
55 CONTINUE
C *** READ LOAD CONTROL CARDS
  READ(5,900) DJMMY
  DO 80 I=1,IMASSN
    IF(IMASSN.EQ.I) GO TO 81
    READ(5,900) DUMMY
80 CONTINUE
  IF(IDAMPN.EQ.0) GO TO 91
  DO 90 I=1,IDAQN
    READ(5,900) DUMMY
90 CONTINUE
91 READ(5,9002) FCON
  IF(FCON.EQ.0) GO TO 100
  CARDNR=NEQ/6.0

```



```

TEST=CARONR-NCARD
IF(TEST.GT.0) NCARD=NCARD+1
NCARD=NCARD+2
20 CONTINUE
C *** READ MATERIAL PROPERTIES
DO 50 J=1,NUMMAT
  DO 45 I=1,NCARD
    READ(5,9000) DUMMY
  45 FORMAT(20A4)
  50 CONTINUE
C *** READ OR GENERATE ELEMENT DATA CARDS
  IF(NPAR(14).EQ.0) NPAR(14)=1
  NEL=NPAR(14)-1
  READ(5,9002) IVEL(IJ,IINC)
  130 READ(5,9005) X15
  9002 FORMAT(3I5,20X,I5)
  IF(INC.EQ.0)
    INC=1
  140 NEL=NEL+1
  ML=INEL-NEL
  IF(ML) 150,155,160
  150 CALL ERROR(7)
  155 I=I
C *** NO GENERATION OF NODE POINTS REQUIRED
  C 155 J=JJ
  GO TO 162
C *** GENERATION OF NODE POINTS REQUIRED
  C 160 I=I+KN
  J=J+KN
  162 CONTINUE
  NUMEL=NUMEL+1
  WRITE(10,N2) IVEL(IJ,J)
  IF(VEL.EQ.NPAR(2)) RETURN
  IF(NEL.LT.INEL) GO TO 140
  KN=IINC
  GO TO 130
END
SUBROUTINE ADPLAN
COMMON/GCONT/NUMNP,NPAR(20),NLTYP,NUMEL
DIMENSION NP(12),INP(8)

CUCUCUCUCU
C *** THIS SUBROUTINE TO READ ADINA 8-NODE PLANE ELEMENT DATA
*** IT IS CALLED BY ELTYPE
CUCUCUCUCU
CUCUCUCUCU
ADNA1630
ADNA1640
ADNA1650
ADNA1660
ADNA1670
ADNA1680
ADNA1690
ADNA1700
ADNA1710
ADNA1720
ADNA1730
ADNA1740
ADNA1750
ADNA1760
ADNA1770
ADNA1780
ADNA1790
ADNA1800
ADNA1810
ADNA1820
ADNA1830
ADNA1840
ADNA1850
ADNA1860
ADNA1870
ADNA1880
ADNA1890
ADNA1900
ADNA1910
ADNA1920
ADNA1930
ADNA1940
ADNA1950
ADNA1960
ADNA1970
ADNA1980
ADNA1990
ADNA2000
ADNA2010
ADNA2020
ADNA2030
ADNA2040
ADNA2050
ADNA2060
ADNA2070
ADNA2080
ADNA2090
ADNA2100

```

```

C * */
      NUMMAT=NPAR(13) NUMBER OF MATERIAL CASE CARDS
      NSTRES=NPAR(14) NUMBER OF ELEMENT STRESS CARDS
      NCARD=1
      IF(NPAR(15)=EQ.) NCARD=1
      IF(NPAR(15)=EQ.) NCARD=2
      IF(NPAR(15)=EQ.) NCARD=3
      IF(NPAR(15)=EQ.) NCARD=4
      IF(NPAR(15)=EQ.) NCARD=5
      IF(NPAR(15)=EQ.) NCARD=6
      IF(NPAR(15)=EQ.) NCARD=7
      IF(NPAR(15)=EQ.) NCARD=8
      IF(NPAR(15)=EQ.) NCARD=9
      IF(NPAR(15)=EQ.) NCARD=10
      IF(NPAR(15)=EQ.) NCARD=11
      IF(NPAR(15)=EQ.) NCARD=12
      IF(NPAR(15)=EQ.) NCARD=13
      IF(NPAR(15)=EQ.) NCARD=14
      IF(NPAR(15)=NE.) NCARD=1
      IF(NPAR(15)=NE.) NCARD=2
      IF(NPAR(15)=NE.) NCARD=3
      IF(NPAR(15)=NE.) NCARD=4
      IF(NPAR(15)=NE.) NCARD=5
      IF(NPAR(15)=NE.) NCARD=6
      IF(NPAR(15)=NE.) NCARD=7
      IF(NPAR(15)=NE.) NCARD=8
      IF(NPAR(15)=NE.) NCARD=9
      IF(NPAR(15)=NE.) NCARD=10
      IF(NPAR(15)=NE.) NCARD=11
      IF(NPAR(15)=NE.) NCARD=12
      IF(NPAR(15)=NE.) NCARD=13
      IF(NPAR(15)=NE.) NCARD=14
      IF(NPAR(15)=GT.0.) NCARD=NCARD+1
      NCARD=INT(CARDNR-NCARD)
      TEST=CARDNR-NCARD
      IF(TEST.GT.0.) NCARD=NCARD+1
      CONTINUE
      N12=12
      READ(MATERIAL PROPERTIES
      DO 50 J=1,NUMMAT
      READ(5,9000) DUMMY
      9000 FORMAT(20A4)
      DO 45 I=1,NCARD
      READ(5,9000) DUMMY
      45 CONTINUE
      50 READ(STRESS OUTPUT TABLE CARDS
      DO 50 J=1,NUMMAT
      READ(5,9000) DUMMY
      9000 FORMAT(20A4)
      DO 60 I=1,NCARD
      READ(5,9000) DUMMY
      60 CONTINUE
      61 CONTINUE
      READ AND GENERATE ELEMENT DATA CARDS
      IF(NPAR(14)=EQ.0) NPART(14)=1
      NEL=NPAR(14)-1
      130 READ(5,9002) NEL,INC
      IF(INC.EQ.0) INC=1
      9002 FORMAT(15I5X,15)
      READ(5,9004)(INP(I),I=1,8)
      9004 FORMAT(8I5)
      140 NEL=NEL+1
      ML=INEL-NEL
      150 IF(ML>150) 155,160
      C ** NO GENERATION OF NODE POINTS REQUIRED

```

```

155 DO 156 I=1,4
156 I=I+4
157 I=I+8
158 NP(1)=INP(1)
NP(15)=0
NP(19)=INP(15)
CONTINUE
159 GO TO 162 GENERATION OF NODE POINTS REQUIRED
160 DO 161 I=1,N12
IF (NP(1)EQ.0) GO TO 161
NP(1)=NP(1)+KN
CONTINUE
161 CONTINUE
NUMEL=NUMEL+1
WRITE(10) N12,NEL,(NP(I),I=1,N12)
IF(NEL.EQ.NPAR(2)) RETURN
IF(NEL.LT.INEL) GO TO 140
KN=INEL
GO TO 130
END
SUBROUTINE AD3DEE
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C *** THIS SUBROUTINE TO READ ADINA 3-D SOLID ELEMENT DATA
C *** THIS ROUTINE CALLED BY ELTYPE
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C COMMON/GCONT/YUMNP,NPAR(20),NELTYP,NUMEL
C DIMENSION NP(20),INP(20)
C NUMMAT=NPAR(16)
C NSTRES=NPAR(13)
C CALCULATE THE NUMBER OF MATERIAL CASE CARDS
C IF(NPAR(15)=0) NCARD=1
C IF(NPAR(15)=1) NCARD=2+NPAR(18)
C IF(NPAR(15)=2) NCARD=3
C IF(NPAR(15)=3) NCARD=4
C IF(NPAR(15)=4) NCARD=5
C IF(NPAR(15)=5) NCARD=2
C IF(NPAR(15)=6) NCARD=1
C IF(NPAR(15)=7) NCARD=1
C IF(NPAR(15)=8) NCARD=6
C IF(NPAR(15)=9) NCARD=6
C IF(NPAR(15)=10) NCARD=6
C IF(NPAR(15)=11) NCARD=6
C IF(NPAR(15)=12) NCARD=6
C CARDNR=NPAR(17)/8.0
C CARD=INT(CARDNR)
C TEST=CARDNR-NCARD

```

```

      IF(TEST.GT.0.1) NCARD=NCARD+1
  20 CONTINUE
      N20=20
      READ(MATERIAL PROPERTIES
      DO 50 J=1,NUMMAT
        READ(5,9000) DUMMY
  50 FORMAT(20A4)
      DO 45 I=1,NCARD
        READ(5,9000) DUMMY
  45 CONTINUE
      50 CONTINUE
      READ STRESS OUTPUT TABLE CARDS
      IF(INPAR(13).EQ.0) GO TO 61
      DO 60 I=1,N_STRES$ DUMMY
  60 READ(5,9000) DUMMY
      60 CONTINUE
      61 CONTINUE
      IF(INPAR(14).EQ.0) NPAR(14)=1
      NEL=NPAR(14)-1
      130 READ(5,9002) IVEL,IINC
      9002 FORMAT(15,3X,15,I1)
      IF(IINC.EQ.0) IINC=1
      READ(5,9004) (INP(I),I=1,8)
      READ(5,9004) (INP(I),I=9,N20)
  9004 FORMAT(12I5)
      140 NEL=NEL+1
      IF(ML.NE.NEL) 150,160
  150 CALL ERROR(91)
      C *** NO GENERATION OF NODE POINTS REQUIRED
      155 DO 156 I=1,N20
        NP(I)=INP(I)
  156 CONTINUE
      156 GO TO 162
      C *** GENERATION OF NODE POINTS REQUIRED
      160 DO 161 I=1,N20
        IF(NP(I).EQ.0) GO TO 161
        NP(I)=NP(I)+KN
      161 CONTINUE
      162 NUMEL=NUMEL+1
      WRITE(10,N20,'(NP(I),I=1,N20)')
      IF(NEL.EQ.NPAR(2)) RETURN
      IF(NEL.LT.NEL) GO TO 140
      KN=IINC
      GO TO 130
      END
      SUBROUTINE ADSEAM

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```

C * * * * * THIS SUBROUTINE TO READ ADINA 2NODE BEAM ELEMENTS
C * * * * * THIS ROUTINE CALLED BY ELTYPE
C * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * * *
C COMMON/GCONT/NUMNP,NPAR(20),NELTYP,NUMEL
C N2=2
C NUMMAT=NPAR(15)
C IF(NUMMATERIAL.EQ.3) NUMMAT=1
C READ(MATERIAL,PROPERTIES
C DO 50 J=1,NUMMAT
C     DO 45 I=1,2
C        READ(15,9000) DUMMY
C 9000 FORMAT(20A4)
C 45 CONTINUE
C 50 CONTINUE
C *** READ STRESS OUTPUT TABLE CARDS
C IF(NPAR(13).EQ.0) GO TO 81
C IF(NPAR(14).EQ.0) NPAR(14)=16
C NST=NPAR(13)
C NCDSST=NPAR(14)/16.0
C NCDSST=INT(CARDST)
C TEST=CARDST-NCDSST
C IF(TEST.GT.0.01) NCDSST=NCDST+1
C NST=NST*NCDSST
C DO 80 I=1,NST
C     READ(15,9000) DUMMY
C 80 CONTINUE
C 81 CONTINUE
C *** READ OR GENERATE ELEMENT DATA CARDS
C IF(NPAR(17).EQ.0) NPAR(17)=1
C NEL=NPAR(17)-1
C 130 READ(15,9002) IVEL(1),JJ,IINC
C 9002 FORMAT(15X,15) IVEL(1),JJ,IINC
C IF(IINC.EQ.0) IINC=1
C 140 NEL=NEL-1
C ML=INEL-NEL
C IF(ML.LT.155) 160
C 150 CALL ERROR(10)
C 155 C *** NO GENERATION OF NODE POINTS REQUIRED
C           I=1
C           J=J+1
C           GO TO 162
C *** GENERATION OF NODE POINTS REQUIRED
C 160

```

162
J=J+KN
CONTINUE
NUMEL=NUMEL+1
WRITE(101,N2,VELL)I,J
IF(INEL.EQ.NPAR(2))RETURN
IF(INEL.LT.INEL)GO TO 140
KN=INC
GO TO 130
END

ADNA4030
ADNA4040
ADNA4050
ADNA4060
ADNA4070
ADNA4080
ADNA4090
ADNA4100
ADNA4110

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